Adur & Worthing Level 2 Strategic Flood Risk Assessment – site sheets

JBA

Final report

July 2020

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Adur District Council & Worthing Borough Council



JBA Project Manager

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Revision History

Revision Ref/Date	Amendments	Issued to
Version 1 / June 2020	Draft Report	Ian Moody (Adur District Council & Worthing Borough Council)
Version 2 / July 2020	Final Report	Ian Moody (Adur District Council & Worthing Borough Council)

Contract

This report describes work commissioned by Ian Moody of Adur District Council & Worthing Borough Council, by an email dated 7 June 2019. Ed Hartwell, Kate Fairfield and Anna Hastings of JBA Consulting carried out this work.

JBA

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Purpose

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Acknowledgements

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- Adur District Council & Worthing Borough Council
- West Sussex County Council
- The Environment Agency
- Southern Water

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	A Sile leieleille	BO08193 / HBO0017				
Site name		Adur Civic Centre Site				
	Existing watercourses	There are no watercourses within the site boundary, however the River Adur (Main River) flows from west to east approximately 170m south of the site.				
	Flood history	There are no recorded flood events within the site.				
		(proportion reported a between larger or small Percentages round	Proportion of the site at r re for the area of land occu ler return period events, and ed to the nearest 1%. Area	i sk pied by each flood extent d therefore not cumulative. s <0.5% not recorded)		
		5% AEP	0.5% AEP	0.1% AEP		
		0%	100%	0%		
	Coastal / tidal	 Available modelled data: The site is covered by the Environment Agency River Adur (Fluvial/Tidal) 2018 Flood Modeller-TUFLOW model. The model was updated by JBA Consulting as part of the Shoreham Tidal Walls modelling project. The extent of the Flood Zones predicted by the flood model are also the extent of the actual flood risk, as there are no flood risk management features that change the risk. Flood characteristics: The site is predicted to be at risk from coastal flooding due to the proximity of the tidal River Adur to the south. The entire site is at risk of flooding during a 0.5% AEP 				
Sources of		Proportion of site at risk (RoFSW)				
flood risk		between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)				
		Percentages round	ed to the nearest 1%. Area	s <0.5% not recorded)		
		3.3% AEP	ed to the nearest 1%. Area 1% AEP	s <0.5% not recorded) 0.1% AEP		
		3.3% AEP	ed to the nearest 1%. Area 1% AEP 6%	s <0.5% not recorded) 0.1% AEP 41%		
	Surface Water	Description of surface w 0% Description of surface w The site is at a very low boundaries during the 3.3' a 6% increase in flood e event. In the 0.1% AEP evalled all edges of the site, imp presence of the existing between the existing buildings on the site is a secount of existing buildings on the site is greater than 0.57	ed to the nearest 1%. Area 1% AEP 6% rater flow paths: risk of surface water floodi % AEP rainfall event (less t xtent along the northern by yent there is a further 41% in boacting just under half of t building in the mapping is l building footprints so the f site. It also only considers 5.	s <0.5% not recorded) 0.1% AEP 41% ng along the north and east han 1% of the site). There is bundary during the 1% AEP crease in flood extent around he site (47%), although the ikely to have impacted flood lood risk may be affected by flood risk where the hazard		
	Surface Water	Percentages round 3.3% AEP 0% Description of surface w The site is at a very low boundaries during the 3.3' a 6% increase in flood event. In the 0.1% AEP evall edges of the site, impresence of the existing between the exist exists and the exists and the exists and the exist exists and the exists	ed to the nearest 1%. Area 1% AEP 6% vater flow paths: risk of surface water floodi % AEP rainfall event (less t xtent along the northern be vent there is a further 41% in bacting just under half of t building in the mapping is l building footprints so the f site. It also only considers 5. bk in JBA Groundwater Ma	s <0.5% not recorded) 0.1% AEP 41% ng along the north and east han 1% of the site). There is bundary during the 1% AEP crease in flood extent around he site (47%), although the ikely to have impacted flood lood risk may be affected by flood risk where the hazard		
	Surface Water Groundwater	3.3% AEP 0% Description of surface w The site is at a very low boundaries during the 3.3' a 6% increase in flood e event. In the 0.1% AEP evall edges of the site, imp presence of the existing b extents. RoFSW takes account of existing buildings on the site is greater than 0.57 Proportion of site at rist Depth below surface 0-0.025m	ed to the nearest 1%. Area 1% AEP 6% vater flow paths: risk of surface water floodi % AEP rainfall event (less t xtent along the northern be vent there is a further 41% in bacting just under half of t building in the mapping is l building footprints so the f site. It also only considers 5. K in JBA Groundwater Ma Depth below surface 0.025-0.5m	s <0.5% not recorded) 0.1% AEP 41% Ing along the north and east han 1% of the site). There is bundary during the 1% AEP crease in flood extent around he site (47%), although the ikely to have impacted flood lood risk may be affected by flood risk where the hazard ap 1% AEP risk categories Total in highest risk categories		



SHLAA / HELA	A site reference	BO08193 / HBO0017	
Site name		Adur Civic Centre Site	
		The site has a medium to low risk of groundwater flooding, with groundwater levels predicted to be between 0.5 and 5m below the surface across the entire site during a 1% AEP groundwater flood event. However, the site level is below the maximum present-day tidal level and therefore groundwater levels may be influenced by the tide.	
		Tidal Groundwater Risk Zone (maximum risk)	Tidal Drainage Risk Zone (maximum risk)
		GW2	SW4
	Tidal Risk Zones	The site is entirely located within Tidal Groundwater Risk Zone GW2. This is to site being situated below the maximum present-day tidal level. Additionally site is also located within an area of medium groundwater flood risk where groundwater levels are more than 0.5m below the surface during a 1% AEP groundwater flood event. The centre of the site is located in Tidal Drainage Risk Zone SW2. This is d this area of the site being located below the maximum present-day tidal lever at a negligible risk from surface water flooding during the 1% AEP surface event. The site is also partially located in Tidal Drainage Risk Zone SW3 i north and south. This is due to these parts of the site being located below present-day tidal level and at risk from surface water flooding in the future. A proportion of the site along the eastern boundary is located within Tidal Drai Risk Zone SW4. This is due to this area being located below the present-day level and is at risk during the 1% AEP surface water flood event.	
	Reservoir	The site is not at risk of reservoir flooding.	

SHLAA / HELAA site reference

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BO08193 / HBO0017

Site name		Adur Civic Centre Site					
	Defences	Defence T	уре	Stan	dard of Protection	Condition	
		The site is currently not protected by defences. Therefore, the defended and undefended modelled flood extents are the same.					
Flood risk management infrastructure		Culvert / structure blockage?	9	There vicinit	There are no known culverts or structures in the vicinity of the site.		
	Residual risk	Impounded water failure?	body	The s breac	ite is not at risk of floo h.	oding due to reservoir	
		Defence breach / overtopping?		The solution	ite is not at risk fror opping.	m defence breach or	
	Flood warning	The site is situated (065FWC3002) Flo of Shoreham, Land	within the Env bod Warning / bing and South	vironme Area ar hwick' (nt Agency's 'Shoreha Id the Environment A 065WAC409) Flood A	m Town and Lancing' gency's 'Inland areas Alert Area.	
Emergency		Dry access and eg events from the we	ress could be est via the A25	availat 59.	ble to the site during a	all surface water flood	
planning Access and egress	Dry access and egress is also available from the west for the 5% AEP coastal/tidal event. For the 0.5% AEP and 0.1% AEP coastal/tidal events dry access and egress would not be available. However, wet access and egress could be available via the same route. During both events, the access route has a hazard rating of 0.75-1.25. This hazard is classified as 'danger for some', generally placing only the most vulnerable people in danger when walking through floodwater.						
		Proportion of site at 0.5% AEP coastal / tidal flood risk					
	Climate change allowances for	Coastal region	Present o	day	Higher Central	Upper End	
	the '2115 EPOCH' (2017 base year)	South Foot	n/a		+0.84m	+1.12m	
	baoo youry	South East	100%		100%	100%	
Climate Change	Implications for Climate the site Change	An increase in flood extent is not predicted for climate change allowances as the site is already located within Flood Zone 3a. However, a significant increase in flood depths will occur. Flood depths for the Higher Central climate change allowance are approximately 0.8m higher across the site than that of the 0.5% AEP event. For the Upper End allowance this difference increases to approximately 1m. Therefore, climate change is predicted to impact the proposed site.					
	Impact of climate	Propo	ortion of site	at 1% A	EP surface water fl	ood risk	
	change on risk from surface	Present day	+20% rai uplif	nfall t	+30% rainfall uplift	+40% rainfall uplift	
	water	6%	13%		18%	22%	
	Implications for the site	Moderate increase predicted for the p extents do not rea increases are loca	es in flood ex lus 20%, 30% ach that of th ted along the	tent of 6 and 4 e 0.1% norther	the 1% AEP surface 0% climate change e AEP surface water n and eastern site bo	water flood event is events. However, the flood extent. These pundaries. Therefore,	

the site will be at a slightly higher risk from surface water flooding in the future.



Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT



Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT









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Site name	Adur Civic Centre Site, Ham Road	Adur and Worthing Level 2	ADUR & WORTHING	JBA
Site area (ha)	0.64	Strategic Flood Risk Assessment Site Summary Sheet mapping	COUNCILS	consulting
All maps: Contains Ordnance Survey data © Crown copyright and database right 2020. Contains public sector information licensed under the Open Government License v3.0. Tidal Groundwater Risk Zones Map: Contains JBA data © JBA Consulting. 2020. Some of the responses contained in this mapping are based on data and information provided by the Natural Environment Research Council (NERC) or its component body the British Geological Survey (BGS). Your use of any information contained in this mapping is at your own risk. Neither JBA, NERC or BGS give any warranty, condition or representation as to the quality,				



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SHLAA / HELA	A site reference	BB08204				
Site name		Beach Green Car Park Shoreham, Beach Green				
		There are no watercourse	s within the site boundary, h	owever an Ordinary		
	Existing watercourses	Watercourse flowing from west to east originating from the River Adur, is situated approximately 70m north of the site. The River Adur is located 310m north east of the site.				
	Flood history	There are no recorded flood events within the site.				
			Proportion of the site at ri	sk		
		(proportion reported a between larger or small Percentages round	re for the area of land occup ler return period events, and ed to the nearest 1%. Area	bled by each flood extent therefore not cumulative. < <0.5% not recorded)		
		5% AEP	0.5% AEP	0.1% AEP		
		7%	22%	13%		
	Coastal / tidal	Available modelled data Adur (Fluvial/ Tidal) 201 updated by JBA Consultin The extent of the Flood Z extent of the actual flood change the risk.	Environment Agency River V model. The model was idal Walls modelling project. model are different from the management features that			
O autor of		Flood characteristics: The site is predicted to be at risk from tidal flooding associated with the River Adur to the north east. The beach green play space east of the site is partially at risk from the 5% AEP tidal flood event. Risk is further increased for the 0.5% AEP tidal event where over a quarter of the site is at risk. For the 0.1% AEP tidal event the entire eastern section of the site is at risk.				
Sources of		Proportion of site at risk (RoFSW)				
noou nak	Surface Water	(proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)				
		3.3% AEP	1% AEP	0.1% AEP		
		0%	3%	42%		
		Description of surface water flow paths: The site is at a negligible risk of surface water flooding for the 3.3% AEP flood event. Small areas along the southern boundary of the site (3%) are at risk during the 1% AEP rainfall event. For the 0.1% AEP event, risk increases to 45% with the majority of the east of the site (Beach Green Play Space) at risk during this event. RoFSW takes account of building footprints so the flood risk may be affected by existing buildings on the site. It also only considers flood risk where the hazard rating is greater than 0.575.				
		Proportion of site at ris	k in JBA Groundwater Ma	p 1% AEP risk categories		
		Depth below surface 0-0.025m	Depth below surface 0.025-0.5m	Total in highest risk categories		
	Groundwater	0%	0%	0%		
		0%0%Almost the entire site is at a negligible risk of groundwater flooding of AEP groundwater flood event. There is a small area located in the corner of the site which has a medium risk of groundwater flooding. I AEP groundwater event, it is predicted that this area would have ground between 0.5m and 5m below the ground surface.				



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SHLAA / HELAA site reference		BB08204		
Site name		Beach Green Car Park Shoreham, Beach Green		
		Tidal Groundwater Risk Zone (maximum risk)	Tidal Drainage Risk Zone (maximum risk)	
		GW2	SW4	
	Tidal Risk Zones	The site is located entirely within Tidal Gr to site predominately being situated below Additionally, the site is also located within more than 0.5m below the surface during The western part of the site is mostly loca SW2. This is due to this area of the site b and the future tidal level and at a negligib the 1% AEP surface water event. A small in the east and south is located within Tid to these areas being located between the at risk during the 1% AEP surface water e east of the site is located in Tidal Drainag area being at risk during the 1% AEP surf situated below present-day tidal level.	oundwater Risk Zone GW2. This is due v the present-day tidal level. an area where groundwater levels are a 1% AEP groundwater flood event. AEP groundwater flood event le risk from surface water flooding during I section of the site along the boundary al Drainage Risk Zone SW3. This is due present-day and future tidal levels and event. Finally, a small area in the south e Risk Zone SW4. This is due to the face water flood event and is also	
	Reservoir	The site is not at risk of reservoir flooding		



SHLAA / HELA	A site reference	BB08204				
Site name		Beach Green Car Park Shoreham, Beach Green				
		[
		Defence T	уре	Stan	dard of Protection	Condition
	Defences	High grou	Ind		2% AEP	Good
		Approximately 10m north of the site is an area of high ground which lines Shoreham Harbour. The Harbour is lined on both sides, running for 500m on the southern side which covers the entire length of the site				
Flood risk management		Culvert / structure blockage?	9	There vicinit	are no known culver y of the site.	ts or structures in the
infrastructure		Impounded water failure?	body	The s breac	ite is not at risk of floo h.	oding due to reservoir
	Residual risk		Defence breach / overtopping?		Due to the presence of flood risk management infrastructure, the site could be at risk from defence breach or overtopping. By using the undefended 1% AEP tidal event as an indicator for breach, the entire site could be at risk if a breach ware to easure	
	Flood warning	The site is situated (065FWC3002) Flo of Shoreham, Lanc	within the Env bod Warning <i>A</i> sing and South	/ironme Area an nwick ((nt Agency's 'Shoreha d the Environment A 065WAC409) Flood A	m Town and Lancing' gency's 'Inland areas lert Area.
Emergency planning	Access and egress	Dry access and egress to the site could surface water flood events and all tidal access and egress would be cut off in th wet access and egress could still be a most likely access route has a hazard 'danger for some'. This generally me would be in danger when walking through		Id be available for the 3.3% AEP and 1% AEP al events from the east via Beach Green. Dry the 0.1% AEP surface water event. However, e available for some via Beach Green. This rd rating of 0.75-1.25, which is classified as means that only the most vulnerable people ough this floodwater.		
		Proportion of site at 0.5% AEP coastal / tidal flood risk				
	Climate change allowances for	Coastal region	Present d	lay	Higher Central	Upper End
	the '2115 EPOCH' (2017		n/a		+0.84m	+1.12m
	base year)	South East	29%		100%	100%
Climate Change	Implications for the site	There is a significant increase in flood extent for both climate change allowances in comparison to the present day. For climate change scenarios, the flood extent covers the entire site for both scenarios and therefore, climate change is predicted to have a large impact on the proposed site.			change allowances in rios, the flood extent e change is predicted	
	Impact of climate	Propo	ortion of site	at 1% A	AEP surface water flo	ood risk
	change on risk from surface	Present day	+20% rai uplift	nfall	+30% rainfall uplift	+40% rainfall uplift
	water	3%	8%		12%	15%
Implications for the site	An increase in flood extent is predicted for surface water 20%, 30% and 40% climate change allowances. However, the extents do not reach that of the 0.1% AEP surface water flood event. These increases are located within the east of the site. Therefore, the site will be at a higher risk from surface water flooding in the future.					



SHLAA / HELAA site reference		BB08204		
Site name		Beach Green Car Park Shoreham, Beach Green		
	Bedrock Geology	The entire site's bedrock geology consists of Newhaven Chalk Formation (chalk).		
	Superficial Geology	The entire site is overlain with Tidal Flat Deposits (clay, silt, s	and and gravel.	
	Soils	The site has loamy and clayey soils of coastal flats with natru groundwater.	ally high	
	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.		
	Historic Landfill Site	There are no historic landfill sites in the vicinity of the site.		
		Implementation of SuDS at the site could provide opportunitie benefits including volume control, water quality, amenity and could provide wider sustainability benefits to the site and surr	es to deliver multiple biodiversity. This ounding area.	
	Broad scale assessment of possible SuDS	Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development.		
Requirement for drainage control and impact		All forms of source control are likely to be appropriate. Whilst controlling run-off from proposed development must be addressed there is also a need to consider the effect of proposals on surface water flows such that predicted surface water flooding is not exacerbated at existing adjacent development.		
mitigation		Infiltration techniques maybe appropriate. Although mapping suggests a low risk of groundwater flooding the site is located below the present day maximum tidal level and may be susceptible to tidally influenced groundwater. Site investigations must be carried out to assess potential for drainage by infiltration.		
		Mapping suggests that the site slopes make it possible to consider most forms of detention.		
		All filtration techniques are likely to be appropriate, subject to confirming that the underlying soils have appropriate seepage and storage capacity. If the site has contamination issues; a liner will be required.		
		All forms of conveyance are likely to be appropriate, although open forms of conveyance may be required, depending upon means of surface water disposal. Where the slopes are >5% features should follow contours or utilise check dams to slow flows. If the site has contamination issues; a liner will be required.		
		There are no public surface water sewers in the area and disc sewer would not be acceptable.	charge to the foul	
	Cumulative	Water Framework Directive Catchment	Sensitivity to cumulative impacts	
	development	River Adur (not part of a river water basin catchment)	Medium	

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT



SHLAA / HELAA site reference		BB08204			
Site name		Beach Green Car Park Shoreham, Beach Green			
	Sequential Test and Exponition Test requirements				
Site name	Sequential Test an The Sequential Test Exception test is app The Exception test is If More vulu If Essential Development will no Highly vuln Highly vuln Recommendations for developers Flood risk assessm At the planning as developmen o may b more v o is on la draina o is on la in futu Other sources of including surface	Beach Green Car Park Shoreham, Beach Green d Exception Test requirements st must be satisfied based on fluvial and other sources of flood risk before the blied. will be required in the following scenarios: nerable or Essential Infrastructure development is proposed to be located in FZ3. Infrastructure is proposed to be located in FZ3b. t be permitted in the following scenarios: erable development within FZ3a. erable, More vulnerable and/ or Less vulnerable development within FZ3b. for requirements of site-specific Flood Risk Assessment, including guidance nent: application stage, a site-specific flood risk assessment will be required for this site t will be located within Flood Zone 3. It will also be required where development: e subject to other sources of flooding, where the development would introduce a ulnerable use; and which has been identified by the Environment Agency as having critical ge problems; or and identified in the strategic flood risk assessment as being at increased flood risk re. of flooding must be considered as part of any site-specific flood risk assessment, e water and groundwater			
ations for Local Plan policy	 Climate change development ar Where there is combination it is Consultation wi be undertaken a Proposals will n 	we water and groundwater. a modelling should be undertaken using the relevant allowances for the type of a level of risk. a reasonable likelihood of multiple sources of flood risk having significant impact in s recommended that consideration is given to assessing the combined risks of these. th the Local Authority, Lead Local Flood Authority and Environment Agency should at an early stage. eed to demonstrate that users will be safe and more vulnerable use is located outside			
	 Guidance for site of New developm example, by: Reduct Reduct Reloca Creatii Safe access and as there is a risk safe access and All development impact flooding SuDS should b green infrastruct Example featur and permeable Assessment of Efforts should b 	design and making development safe: ent must seek opportunities to reduce overall level of flood risk at the site. For ing volume and rate of runoff ating development to zones with lower flood risk ng space for flooding. Id egress should be demonstrated in the 0.5% AEP plus climate change event and k of surface water flooding on the site, consideration should also be given to providing d egress during surface water flood events. It should adopt source control SuDS techniques to reduce the risk of frequent low due to post development runoff. e designed to deliver multiple benefits including water quality, biodiversity, amenity, sture etc. es include swales, attenuation features, green roofs, rainwater capture and reuse paving. runoff should include allowances for climate change effects. e made to limit runoff to greenfield rates and discharge rates from the site should not			



SHLAA / HELAA site reference BB08204		BB08204
Site name		Beach Green Car Park Shoreham, Beach Green
	 SuDS design n Statutory Techr Green infrastru from potential d Further details webpage <u>https</u> water drainage 	nust follow West Sussex County Council guidance, meet the Defra National Non- nical Standards, and follow current best design practice (CIRCA Manual 2015). cture should be considered within the mitigation measures for surface water runoff levelopment. regarding Adur and Worthing Council requirements are available on the following s://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms. A surface e checklist is also available on this webpage. This clearly sets out the LPA's

Site name	Beach Green Car Park Shoreham, Beach Green	Adur and Worthing Level 2	ADUR & WORTHING		
Site area (ha)	0.93	Strategic Flood Risk Assessment Site Summary Sheet mapping	COUNCILS	consult	

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Cito nomo	Beach Green Car Park Shoreham,
Site name	Beach Green

0.93

Site area (ha)

Adur and Worthing Level 2 Strategic Flood Risk Assessment Site Summary Sheet mapping



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SHLAA / HELAA site reference	WB08046
Site name	Bus Depot, Library Place (Also known as Stagecoach, Marine Parade)

	Existing watercourses	There are no watercourses in the vicinity of the site.			
	Flood history	There are no recorded flo	od events within the site.		
		Proportion of the site at risk (proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative Percentages rounded to the nearest 1%. Areas <0.5% not recorded)			
		5% AEP	0.5% AEP	0.1% AEP	
		6% 15% 24% Available modelled data: The site is covered by the Environment Agency Arun to Adur (Coastal) 2016 SW model. The extent of the Flood Zones predicted by the flood model are also extent of the actual flood risk, as there are no flood risk management features change the risk. Flood characteristics: The site is predicted to be at risk from coastal flooding due to the proximity or sea to the south of the site. • A small section of the site along the east and southern boundarie located within the 5% AEP flood extent (approximately 6%). • A further 15% in the east, south and west of the site is located within 0.5% AEP flood extent. • Finally, a further 24% of the site is located within Flood Zone 2, cover are not in the out the provide and points.			
	Coastal / tidal				
Sources of					
fland whate		Proportion of site at risk (RoFSW) (proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)			
tiood risk		proportion reported a between larger or smal Percentages round	oportion of site at risk (Rof are for the area of land occup ler return period events, and led to the nearest 1%. Areas	FSW) ied by each flood extent therefore not cumulative. s <0.5% not recorded)	
tiood risk		(proportion reported a between larger or smal Percentages round 3.3% AEP	oportion of site at risk (Rof are for the area of land occup ler return period events, and led to the nearest 1%. Areas 1% AEP	FSW) ied by each flood extent therefore not cumulative. s <0.5% not recorded) 0.1% AEP	
tiood risk		(proportion reported a between larger or smal Percentages round 3.3% AEP 0%	oportion of site at risk (Rof are for the area of land occup ler return period events, and led to the nearest 1%. Areas 1% AEP 0%	FSW) ied by each flood extent therefore not cumulative. <0.5% not recorded) 0.1% AEP 4%	
TIOOD FISK	Surface Water	(proportion reported a between larger or smal Percentages round 3.3% AEP 0% Description of surface w The majority of the site is water pathways enter the to the south during the 0.7 RoFSW takes account of	oportion of site at risk (Rof are for the area of land occup ler return period events, and ed to the nearest 1%. Areas 1% AEP 0% vater flow paths: at a very low risk of surface of site from Warwick Street in the site from Street in t	FSW) ied by each flood extent therefore not cumulative. (0.5% not recorded) 0.1% AEP 4% water flooding. Two surface he north and Marine Parade ting 4% of the site. bood risk may be affected by	
TIOOD FISK	Surface Water	(proportion reported a between larger or smal Percentages round 3.3% AEP 0% Description of surface w The majority of the site is water pathways enter the to the south during the 0.1 RoFSW takes account of existing buildings on the rating is greater than 0.57	oportion of site at risk (Rofer are for the area of land occup ler return period events, and ed to the nearest 1%. Areas 1% AEP 0% vater flow paths: at a very low risk of surface of site from Warwick Street in the from Warwick Street in the from Warwick Street in the from Street in the form	FSW) ied by each flood extent therefore not cumulative. ied by each flood extent therefore not cumulative. ied by each flood extent ied by each flood extent ied by each flood extent ied by each flood ied by each flood ied by each flood ied by flood	
TIOOD FISK	Surface Water	(proportion reported a between larger or smal Percentages round 3.3% AEP 0% Description of surface w The majority of the site is water pathways enter the to the south during the 0. RoFSW takes account of existing buildings on the rating is greater than 0.57 Proportion of site at ris	oportion of site at risk (Rofer are for the area of land occup ler return period events, and led to the nearest 1%. Areas 1% AEP 0% vater flow paths: at a very low risk of surface of site from Warwick Street in the the theorem of theorem of the theorem of the theorem of the th	FSW) ied by each flood extent therefore not cumulative. s<0.5% not recorded) 0.1% AEP 4% water flooding. Two surface he north and Marine Parade ting 4% of the site. bood risk may be affected by flood risk where the hazard p 1% AEP risk categories	
TIOOD FISK	Surface Water	(proportion reported a between larger or smal Percentages round 3.3% AEP 0% Description of surface w The majority of the site is water pathways enter the to the south during the 0.4 RoFSW takes account of existing buildings on the rating is greater than 0.57 Proportion of site at ris Depth below surface 0-0.025m	oportion of site at risk (Roferer for the area of land occup ler return period events, and led to the nearest 1%. Areas 1% AEP 0% vater flow paths: at a very low risk of surface of site from Warwick Street in the from Warwick Street in the site from Warwick Street in the site. It also only considers for the site. It also only consite. It also only consite. It also only considers for the site. It	FSW) ied by each flood extent therefore not cumulative. ie<0.5% not recorded)	
TIOOD FISK	Surface Water Groundwater	(proportion reported a between larger or smal Percentages round 3.3% AEP 0% Description of surface w The majority of the site is water pathways enter the to the south during the 0.7 RoFSW takes account of existing buildings on the rating is greater than 0.57 Proportion of site at ris Depth below surface 0-0.025m	oportion of site at risk (Rofer are for the area of land occup ler return period events, and led to the nearest 1%. Areas 1% AEP 0% vater flow paths: at a very low risk of surface of surface of surface of site from Warwick Street in the site from Warwick Street in the site. It also only considers for the site. It also only considers for the surface of surface of surface of surface of surface of site. It also only considers for the surface of sufface of surface of sufface of	SW) ied by each flood extent therefore not cumulative. <0.5% not recorded) 0.1% AEP 4% water flooding. Two surface he north and Marine Parade ting 4% of the site. bood risk may be affected by flood risk where the hazard p 1% AEP risk categories Total in highest risk 8%	



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SHLAA / HELAA site reference		WB08046		
Site name		Bus Depot, Library Place (Also known as	Stagecoach, Marine Parade)	
		Tidal Groundwater Pisk Zono	Tidal Drainage Risk Zone	
		(maximum risk)	(maximum risk)	
		GW2	SW2	
	Tidal Risk Zones	The site is mostly situated within Tidal Gri the site is situated above the current tidal within an area of medium groundwater flo more than 0.5m below the surface during Small sections in the east and south of th Groundwater Risk Zone GW2. The area situated below the existing tidal level and groundwater levels are between 0.5m and groundwater flood event. The area to the situated between the present-day and fut groundwater risk area where groundwate below the surface during a 1% AEP grour The site is mostly located within Tidal Dra being located above the current tidal leve is also at a negligible risk from surface w water event. A small section in the east o Risk Zone 2. This is due to this area beir present-day tidal level, and at a negligible the 1% AEP surface water event.	oundwater Risk Zone 1. This is because level but below the future tidal level and ood risk where groundwater levels are a 1% AEP groundwater flood event. e site is situated within Tidal to the east is in this zone due to being at a medium groundwater risk where d 5m below the surface during a 1% AEP e south is located in zone GW2 as it is ure tidal levels and within a higher r levels are between 0.025m and 0.5m ndwater flood event. inage Risk Zone 1. This is due to the site I but below the future tidal level. The site rater flooding during the 1% AEP surface of the site is situated within Tidal Drainage ng located at a lower elevation, below the e risk from surface water flooding during	
Reservoir		The site is not at risk of reservoir flooding		



SHLAA / HELAA site reference	WB08046
Site name	Bus Depot, Library Place (Also known as Stagecoach, Marine Parade)

	Defences	Defence T	уре	Stan	dard of Protection	Condition	
	Defences	There are no defences within the vicinity of the site.					
Flood risk management		Culvert / structure blockage?		There vicinit	are no known culver y of the site.	<pre>cnown culverts or structures in the site.</pre>	
infrastructure	Residual risk	Impounded water failure?	ter body The site is not at risk of floo breach.		oding due to reservoir		
		Defence breach / overtopping?		The site is not at risk from defence breach or overtopping.			
	Flood warning	The site is situated within the Environment Agency's 'Coastal areas of Rustingt to Shoreham' (065WAC407) Flood Alert Area and the Environment Agenc 'Rustington, Worthing and Lancing' (065FWC2801) Flood Warning Area.				l areas of Rustington nvironment Agency's arning Area.	
Emergency planningDry access and egress could be available to the site during the surface water events to the north of the site via Warwick Str egress would be cut off in the 0.1% AEP event. However we could still be available given the maximum hazard rating of 0 of the site. This generally means that only the most vulnerab danger when walking through this floodwater. Dry access and egress can be available to the site to the nor all coastal flood events.				e 3.33% and 1% AEP eet. Dry access and et access and egress 0.75-1.25 to the north le people would be in th via Bedford Row in			
		Proportion of site at 0.5% AEP coastal / tidal flood risk					
Climate change allowances for Coastal regio	Coastal region	Present da	ay	Higher Central	Upper End		
	the '2115 EPOCH' (2017 base year)	Couth Foot	n/a		+0.84m	+1.12m	
	base year)	South East	15%		100%	100%	
Climate Change	Climate Climate Change Implications for the site Implications for the		ent for both climate c or the climate change present day 0.1% AE is predicted to have s	hange allowances in e scenarios, the flood P event, to affect the significant impact the			
	Import of all mate	Propo	rtion of site a	at 1% AEP surface water flood risk			
	change on risk from surface	Present day	+20% rain uplift	fall	+30% rainfall uplift	+40% rainfall uplift	
	water	0%	Less than	1%	1%	1%	
	Implications for the site	A very slight increa is predicted for the these extents are n extent. These inc Therefore, the site the future.	se in flood ext plus 20%, 30 ot predicted to reases are lo will be at a ma	ent du 0% an o reach cated arginall	ring the 1% AEP surfa d 40% climate chang n that of the 0.1% AE within the south eas y higher risk from sur	ace water flood event je events. However, P surface water flood st corner of the site. face water flooding in	



SHLAA / HELAA site reference		WB08046
Site name		Bus Depot, Library Place (Also known as Stagecoach, Marine Parade)
	Bedrock Geology	The entire site's bedrock geology consists of Lewes Nodular Chalk Formation (chalk).
	Superficial Geology	The entire site is overlain with River Terrace Deposits (undifferentiated), sand, silt and clay.
	Soils	The site has freely draining slightly acid loamy soils.
	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.
	Historic Landfill Site	There are no historic landfill sites in the vicinity of the site
		Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could provide wider sustainability benefits to the site and surrounding area.
		Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development.
Requirement for drainage control and impact mitigation		Most source control techniques are likely to be appropriate. Mapping suggests that permeable paving may have to use non-infiltrating systems across the site given the possible risk from groundwater flooding (medium to high). This must be confirmed via site investigation to assess the potential for infiltration. Whilst controlling run-off from proposed development must be addressed, there is also a need to consider the effect of proposals on surface water flows such that predicted surface water flooding is not exacerbated at existing adjacent development.
	Broad scale assessment of possible SuDS	Infiltration techniques may be appropriate. Mapping suggests a medium risk of groundwater flooding across most of the site, although there is a small area of high risk in the south of the site. Underlying soils may be permeable. Further site investigation must be carried out to assess potential for drainage by infiltration. If infiltration is suitable it should be avoided in southern areas of the site where the depth to the water table is <1m.
		Given the high-density nature of the site, use of SuDS is recommended – urban sites should not preclude the use of SuDS.
		Mapping suggests that the ground slopes on the site would mean it would be possible to consider most forms of detention. A liner maybe required due to the potential groundwater flooding on the site.
		Where there is not a significant risk of groundwater flooding, all filtration techniques are likely to be appropriate, subject to confirming that the underlying soils have appropriate seepage and storage capacity via site investigation works.
		All forms of conveyance are likely to be appropriate. Where the slopes are >5% features should follow contours or utilise check dams to slow flows. A liner maybe required to prevent the ingress of groundwater.

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT



SHLAA / HELAA site reference		WB08046			
Site name		Bus Depot, Library Place (Also known as Stagecoach, Marine	e Parade)		
Cumulative impacts of development		Water Framework Directive Catchment	Sensitivity to cumulative impacts		
		River Adur (not part of a river water basin catchment)	High		
Recommend- ations for Local Plan policy	Sequential Test an The Sequential Test Exception test is application The Exception test is application If Highly vulue If More vulue If Essential Development will note Highly vulue Highly vulue Highly vulue Highly vulue Recommendations for developers Flood risk assessmenter At the planter site if developers Flood risk assessmenter Other sour assessmenter Other sour assessmenter Considerate of higher so opportunity coastal / tic Climate ch developmenter Where there impact in c combined re Orsoultation should be re Proposals outside Floo Guidance for site of New devel	d Exception Test requirements t must be satisfied based on fluvial and other sources of olied. will be required in the following scenarios: Inerable development is proposed to be located in FZ2. herable or Essential Infrastructure development is proposed to infrastructure is proposed to be located in FZ3b. t be permitted in the following scenarios: erable development within FZ3a. erable, More vulnerable and / or Less vulnerable development for requirements of site-specific Flood Risk Assessment, nent: ning application stage, a site-specific flood risk assessment will lopment: located in Flood Zones 2 or 3; subject to other sources of flooding, where the development we ore vulnerable use; on land which has been identified by the Environment Agency ainage problems; or on land identified in the strategic flood risk assessment as bein sk in future. ces of flooding must be considered as part of any site-specific t, including surface water and groundwater. ion should be given to the potential effects of climate change in ea levels on groundwater and surface water. Proposals should to include measures that provide for a reduction in the predict laf flood risk at existing development. ange modelling should be undertaken using the relevant allow nt and level of risk. te is a reasonable likelihood of multiple sources of flood risk has ombination it is recommended that consideration is given to as isks of these. In with the Local Authority, Lead Local Flood Authority and undertaken at an early stage. will need to demonstrate that users will be safe and more vulr od Zone 3b. Besign and making development safe: opment must seek opportunities to reduce the overall level of	flood risk before the be located in FZ3. within FZ3b. including guidance Il be required for this vould introduce a as having critical ng at increased flood flood risk ncluding the impact d consider the ed surface water and vances for the type of aving significant sessing the Environment Agency herable use is located		
	For examp o R o R o C	le, by: educing volume and rate of runoff elocating development to zones with lower flood risk reating space for flooding.			



SHLAA / HELAA site reference		WB08046
Site name		Bus Depot, Library Place (Also known as Stagecoach, Marine Parade)
	 Safe access change everalso be giv All develop low impact SuDS shown amenity, gr Example for reuse and search and	as and egress should be demonstrated in the tidal/coastal 0.5% AEP plus climate ent and as there is a risk of surface water flooding on the site, consideration should en to providing safe access and egress during surface water flood events. Intermet should adopt source control SuDS techniques to reduce the risk of frequent flooding due to post development runoff. Und be designed to deliver multiple benefits including water quality, biodiversity, reen infrastructure etc. eatures include swales, attenuation features, green roofs, rainwater capture and permeable paving. Int of runoff should include allowances for climate change effects. Und be made to limit runoff to greenfield rates and discharge rates from the site should e downstream flood risk. gn must follow West Sussex County Council policy, meet the Defra National Non- echnical Standards, and follow current best design practice (CIRIA C753 Manual astructure should be considered within the mitigation measures for surface water a potential development and consider using Flood Zones 2 and 3 as public open ails regarding Adur and Worthing Council requirements are available on the following <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms</u> . A ter drainage checklist is also available on this webpage. This clearly sets out the irements for avoiding pre-commencement conditions, or to discharge conditions.

Site name Bus Depot, Library Place Adur and Worthing Level 2 ADUR & WORTHING JBA Site area (ha) 0.67 0.67 Site Summary Sheet mapping Image: Council s Image: Counci Ima

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Site name Bus Depot, Library Place Adur and Worthing Level 2 Adur and Worthing Level

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Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT





SHLAA / HELAA site reference		(Part of) WB08138			
Site name		Caravan Club			
		Proportion of the site at risk (proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)			
	Fluvial	0% Available modelled data The site is covered by the Flood Modeller-TUFLOW Adur and Worthing Counc Zones predicted by the flo there are no flood risk ma Flood characteristics: The site is within Flood Z	0% Rife (Fluvial/Tidal) 2019/20 ated by JBA Consulting for RA. The extent of the Flood t of the actual flood risk, as ge the risk.		
		rivers. Pro (proportion reported a between larger or smal Percentages round	oportion of site re for the area ler return period ed to the neare	e at risk (RoF of land occupi l events, and t st 1%. Areas	SW) ed by each flood extent herefore not cumulative. <0.5% not recorded)
		3.3% AEP 0%	1% A	AEP %	0.1% AEP 3%
	Surface Water	 Description of surface water flow paths: The site is at a very low risk of surface water flooding in the north east during the 1% AEP rainfall event (less than 1%). There is a 3% increase in flood extent during the 0.1% AEP event in the east of the site. RoFSW takes account of building footprints so the flood risk may be affected by existing buildings on the site. It also only considers flood risk where the hazard rating is greater than 0.575. 			
		Proportion of site at risk in JBA Groundwater Map 1% AEP risk ca			
	Groupdwater	Depth below surface 0-0.025m	Depth below surface 0.025-0.5m		Total in highest risk categories
		18% The eastern section of the groundwater levels predic AEP groundwater flood e	18% 0% 18% eastern section of the site (18%) has a high risk of groundwater floodi ndwater levels predicted to be less than 0.025m below surface during groundwater flood event. The remainder of the site is at a pedicible		18% f groundwater flooding with below surface during a 1% site is at a negligible risk of
		groundwater flooding. Tidal Groundwater F	Risk Zone	Tidal	Drainage Risk Zone
		(maximum ris	sk)	(1	maximum risk) SW0
	Tidal Risk Zones	The site is entirely locate Drainage Risk Zone SW0 tidal level.	ed within Tidal 9. This is due to	Groundwater o the site beir	Risk Zone GW0 and Tidal g located above the future
	Reservoir	The site is not at risk of reservoir flooding.			

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT

SHLAA / HELAA site reference	(Part of) WB08138
Site name	Caravan Club

JBA consulting

Flood risk management infrastructure	Defences	Defence Type		Stan	Standard of Protection		C	Condition	
		There are no defences within the vicinity of the site. Therefore, the defended and undefended modelled flood extents are the same.							
	Residual risk	Culvert / structure blockage?		There are no known culverts or structures in the vicinity of the site.					
		Impounded water body failure?		The site is not at risk of flooding due to a reservoir breach.					
		Defence breach / overtopping?		The site is not at risk from defence breach or overtopping.					
Emergency planning	Flood warning	The site is not covered by an Environment Agency Flood Alert Area or Flood Warning Area.							
	Access and egress	Dry access and egress could be available to the site during the 3.3% AEP and 1% AEP surface water flood events from the south via Titmore Way. Dry access and egress would not be available during the 0.1% AEP rainfall event. However, wet access and egress could be possible for some via the same route, given the low to medium hazard rating of 0.75-1.25. This generally means that only the most vulnerable people would be in danger when walking through this floodwater. Dry access and egress via Titmore Way would be available for all fluvial flood events.							
Climate Change	Climate change allowances for '2080s'	Proportion of site at 1% AEP fluvial flood risk							
		River Basin District	Present da	у	Central	High Cent	ner tral	Upper End	
		South East	n/a	-	+35% flow uplift	+45% upli	flow ift	+105% flow uplift	
			0%		0%	0%	, 0	0%	
	Implications for the site	The future extent of the 1% AEP event is not predicted to impact the site.							
	Impact of climate change on risk from surface water	Proportion of site at 1% AEP surface water flood risk							
		Present day	+20% rainfall uplift		+30% rainfall uplift		+40% rainfall uplift		
		0%	1%	1%		1%		1%	
	Implications for the site	A very slight increase in flood extent of the future 1% AEP surface water flood events is predicted to occur for the plus 20%, 30% and 40% climate change events. However, they do not reach the 0.1% AEP surface water flood extent. These increases are located in the east of the site. Therefore, the site will be at a marginally higher risk from surface water flooding in the future.							

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT



SHLAA / HELAA site reference		(Part of) WB08138					
Site name		Caravan Club					
Requirement for drainage control and impact mitigation	Bedrock Geology	The entire site's bedrock geology consists of London Clay Formation (clay, sil and sand).					
	Superficial Geology	The majority of the site's superficial geology is Head- Clay, Silt, Sand and The south eastern edge of the site is formed of River Terrace Deposits (undifferentiated).					
	Soils	The site has slowly permeable seasonally wet slightly acid but base-rich loam and clayey soils.					
	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.					
	Historic Landfill Site	There are no historic landfill sites in close proximity to the site.					
	Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could provide wider sustainability benefits to the site and surrounding area.					
		Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development.					
		Most source control techniques are likely to be appropriate. Mapping suggests that permeable paving may have to use non-infiltrating systems on parts of the given the possible risk from groundwater flooding (high in the south east corner). This must be confirmed via site investigations to assess the potential for infiltration techniques across the whole site.					
		Mapping also suggests that slopes may be suitable for selective source control techniques. Whilst controlling run-off from the proposed development must be addressed there is also a need to consider the effect of proposals on surface water flows such that predicted surface water flooding is not exacerbated at existing adjacent development.					
		Mapping suggests that the site slopes make it possible to consider most forms of detention. A liner maybe required due to the potential for groundwater flooding on the site.					
		All filtration techniques are likely to be appropriate, provided site slopes are <5% at the location of the filtration feature, and areas in the north are avoided where depth to water table is >1m, subject to confirming that the underlying soils have appropriate seepage and storage capacity.					
		All forms of conveyance are likely to be appropriate. If the slopes are >5% features should follow contours or utilise check dams to slow flows. A liner maybe required to prevent the ingress of groundwater.					
	Cumulative impacts of development	Water Framework Directive Catchment	Sensitivity to cumulative impacts				
		Ferring Rife	Medium				

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT



SHLAA / HELAA site reference	(Part of) WB08138	
Site name	Caravan Club	
Further det webpage surface wa LPA's requ	Further details regarding Adur and Worthing Council requirements are available on the follow webpage https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms . surface water drainage checklist is also available on this webpage. This clearly sets out LPA's requirements for avoiding pre-commencement conditions, or to discharge conditions.	

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⊐ Metres

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70 □Metres

35 70 □Metres 꼬
Site name	Caravan Club	Adur and Worthing Level 2	ADUR & WORTHING	JBA
Site area (ha)	2.55	Strategic Flood Risk Assessment Site Summary Sheet mapping	COUNCILS	consulting
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SHLAA / HELAA site reference		WB16006		
Site name		Centenary House		
	OS Grid reference	TQ 11766 04353		
	Local Authority	Worthing Borough Council		
	Area	4.11 ha		
	Current land use)ffice use / Police Custody Suite		
	Proposed site use	Mixed use - 100 residential units & 10,000m ² employment floorspace		
	Flood risk vulnerability	More vulnerable		
Site details	Topography	<complex-block> Site Boundary Levation Image: Site Boundary <t< th=""></t<></complex-block>		



SHLAA / HELAA site reference		WB16006					
Site name		Centenary House					
		1					
	Existing watercourses	There are no watercourse	s within the vicinity of the si	ie.			
	Flood history	The Environment Agency's Recorded Flood Outline dataset identifies that the entire site was affected by a drainage flood incident in 1980. A further incident of flooding as a result of the overtopping of defences in 1981 was recorded by the Environment Agency, along the south west boundary of the site, on Littlehampton Road, although it is likely this has been miss-recorded given there are no defences in close proximity to the site.					
			Proportion of the site at ri	sk			
		(proportion reported a between larger or small Percentages round	re for the area of land occup er return period events, and ed to the nearest 1%. Area	bied by each flood extent I therefore not cumulative. s <0.5% not recorded)			
		5% AEP	1% AEP	0.1% AEP			
	Fluvial	0%	0%	0%			
		Flood characteristics: The site is within Flood Zone 1 and is therefore at negligible risk of flooding from rivers.					
	Surface Water	Proportion of site at risk (RoFSW) (proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)					
Sources of		3.3% AEP	1% AEP	0.1% AEP			
flood risk		0%	6%	47%			
		 Description of surface water flow paths: The site is at a very low risk of surface water flooding (1%) along the southern boundary during the 3.3% AEP rainfall event. During the 1% AEP event there is a 5% increase in flood extent, which originates from a surface water flow path along the existing access road in the east before ponding in the centre of the site. In a 0.1% AEP event over half of the site (53%) is at risk of flooding, in particular in the north, south and east. RoFSW takes account of building footprints so the flood risk may be affected by existing buildings on the site. It also only considers flood risk where the hazard rating is greater than 0.575. 					
		Proportion of site at ris	k in JBA Groundwater Ma	p 1% AEP risk categories			
		Depth below surface 0-0.025m	Depth below surface 0.025-0.5m	Total in highest risk categories			
	Groundwater	100%	0%	100%			
		The entire site is at a hig predicted to lie either at o a 1% AEP groundwater flo	h risk of groundwater flood r very near (within 0.025m o ood event.	ing, with groundwater levels of) the ground surface during			

Adur and Worthing Councils

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT

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SHLAA / HELAA site reference		WB16006						
Site name		Centenary House						
		Tidal Groundw (maximu	ater Risk Zo um risk)	one	Tidal Drainage Risk Zone (maximum risk)			sk Zone
		G	N0			S	SW0	
	Tidal Risk Zones	The site is entirely Drainage Risk Zone tidal level.	located with e SW0. This	in Tidal is due t	Groundwat o the site b	er Risk 2 being loca	Zone G ated ab	W0 and Tidal ove the future
	Reservoir	The site is not at risk of reservoir flooding.						
	Defenses	Defence Ty	/pe	Stand	ard of Prot	ection	C	ondition
	Defences	There are no defend	ces within the	e vicinity	of the site.			
Flood risk management		Culvert / structure blockage?		There a vicinity	are no knov of the site.	vn culver	ts or sti	ructures in the
mastructure	Residual risk	Impounded water body failure?		The site breach	e is not at ris	sk of flood	ding due	e to a reservoir
		Defence breach / overtopping?		The site is not at risk from defence breach or overtopping.			nce breach or	
	Flood warning	The site is not covered by an Environment Agency Flood Alert or Flood Warning Area.						
Emergency planning	Access and egress	Dry access and egress could be available to the west of the site via Hildon Close in all surface water and fluvial flood events.						
		Pro	oportion of s	site at 19	% AEP fluv	ial flood	risk	
	Climate change allowances for '2080s'	River Basin District	Present da	ay (Central	High Cent	ner tral	Upper End
		South East	n/a	+3	35% flow uplift	+45% upli	flow ift	+105% flow uplift
			0%		0%	0%	, 0	0%
Climate Change	Implications for the site	The future extent of the 1% AEP event is not predicted to impact the site.					site.	
	Impact of climate	Propor	rtion of site	at 1% Al	EP surface	water flo	ood ris	k
	change on risk from surface	Present day	+20% rai uplift	nfall :	+30% ra uplif	infall t	+4	0% rainfall uplift
	water	6%	17%		24%	D		29%
	Implications for the site	There is a significar and the future 1% A climate change scer surface water event of the site. The site	nt increase in AEP surface marios. Howe These incr will be at a h	flood ex water flo ever, the eases a igher rist	tent betwee od event fo extents do re located p (from surfa	en the ba r the plus not reach predomina ce water	seline 20%, that of antly w floodin	1% AEP event 30% and 40% the 0.1% AEP ithin the south g in the future.



SHLAA / HELAA site reference		WB16006		
Site name		Centenary House		
	Bedrock Geology	The northern section of the site is formed of a bedrock of Lan silt and sand). The southern section of the site is formed of L Formation (clay, silt and sand).	nbeth Group (clay, ondon Clay	
	Superficial Geology	The entire site is overlain with River Terrace Deposits (undiffe	erentiated).	
	Soils	The site has freely draining slightly acid loamy soils.		
	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.		
	Historic Landfill Site	There are no historic landfill sites in close proximity to the site).	
Requirement for drainage control and impact mitigation	Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportunities benefits including volume control, water quality, amenity and could provide wider sustainability benefits to the site and surr Development at this site should not increase flood risk either design of the surface water management proposals should ta impacts of future climate change over the projected lifetime o Most source control techniques are likely to be appropriate. If that permeable paving may have to use non-infiltrating syster risk from groundwater flooding (high). This must be confirme investigations to assess the potential for infiltration. Whilst co proposed development must be addressed there is also a ner effect of proposals on surface water flows such that predicted flooding is not exacerbated at existing adjacent development. Forms of detention may be appropriate provided site slopes a location of the detention feature. A liner maybe required due of groundwater flooding on the site. Filtration techniques may be appropriate in limited areas prov <5% and the depth to the water table is >1m, subject to confii underlying soils have appropriate seepage and storage capace All forms of conveyance are likely to be appropriate. Where the features should follow contours or utilise check dams to slow required to prevent the egress of groundwater.	es to deliver multiple biodiversity. This ounding area. on or off site. The ke into account the f the development. Mapping suggests ns given the possible d via site ontrolling run-off from ed to consider the I surface water are < 5% at the to the potential risk ided site slopes are ming that the city. he slopes are >5% flows. A liner maybe	
	Cumulative impacts of	Water Framework Directive Catchment	Sensitivity to cumulative impacts	
development		Ferring Rife	Medium	

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SHLAA / HELAA site reference	WB16006	
Site name	Centenary House	
SuDS desi Statutory 1 2015).	ign must follow West Sussex County Council policy, meet the Defra National Non- Fechnical Standards, and follow current best design practice (CIRIA C753 Manual	
Green infra runoff from	astructure should be considered within the mitigation measures for surface water potential.	
Further det webpage surface wa LPA's requ	 Further details regarding Adur and Worthing Council requirements are available on the follow webpage <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms</u>. surface water drainage checklist is also available on this webpage. This clearly sets our LPA's requirements for avoiding pre-commencement conditions, or to discharge conditions 	

JBA consulting



Site name	Centenary House	Adur and Worthing Level 2	ADUR & WORTHING	JBA
Site area (ha)	4.11	Strategic Flood Risk Assessment Site Summary Sheet mapping	COUNCILS	consulting
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SHLAA / HELAA site reference		WB08180 and part of WB08045		
Site name		Grafton MSCP and part of Land at 51-93 Montague Street		
	OS Grid reference	TQ 14703 02372		
	Local Authority	Worthing Borough Council		
	Area	0.77 ha		
	Current land use	Car park, bowling alley, retail and service yard		
	Proposed site use	Vixed use - 113 residential units & 2,979m ² of commercial space		
	Flood risk vulnerability	More vulnerable		
Site details	Topography	Site topography Legend Site Boundary Elevation Image: Site Boundary Image: Site Boundary Image: Site Boundary Elevation Image: Site Boundary Image: Site Boundary Image: Site Boundary Itemed under the Open Grown Image: Site Boundary Image: Site Boundary Image: Site Boundary Metres Image: Site Boundary Image: Site Boundary Image: Site Boundary Image: Site Site Site Site Site Site Site Site		



SHLAA / HELA	A site reference	WB08180 and part of WB08045						
Site name		Grafton MSCP and part of	Land at 51-93 Montague S	Street				
	Existing watercourses	There are no watercourses within the vicinity of the site						
	Flood history	There are no recorded floo	There are no recorded flood events within the site					
		(proportion reported a between larger or smal Percentages round	Proportion of the site at r re for the area of land occu ler return period events, and ed to the nearest 1%. Area	isk pied by each flood extent d therefore not cumulative. is <0.5% not recorded)				
		5% AEP	0.5% AEP	0.1% AEP				
		68%	4%	13%				
		Available modelled data	:	-				
	Coastal / tidal	The site is covered by the SWAN model. The extent extent of the actual flood r change the risk.	Environment Agency Arun t of the Flood Zones predict isk, as there are no flood ri	to Adur (Coastal/Tidal) 2016 ted by the model are also the sk management features that				
Sources of flood risk		 Flood characteristics: The site is predicted to be at risk from coastal flooding due to the proximity of the sea to the south of the site. Over two thirds of the site (68%) covering the east, south and centre is located within the 5% AEP flood extent. As the site is already developed it does not currently function as a floodplain and therefore this extent would not be considered to be Flood Zone 3b in policy terms. A further 4% in the west and north east is located within Flood Zone 3a. Finally, a further 13% of the site in the south west is located within Flood Zone 2. 						
		Proportion of site at risk (RoFSW) (proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)						
		3.3% AEP	1% AEP	0.1% AEP				
		0%	5%	17%				
	Surface Water	Description of surface w During a 1% AEP rainfall e water pathways within the pool in the centre of the s pathways during the 0.1%	of flooding along two surface ways flow from the east and se in flood extent along these					
		existing buildings on the site. It also only considers flood risk where the hazard rating is greater than 0.575.						
		Proportion of site at ris	k in JBA Groundwater Ma	ap 1% AEP risk categories				
	Groundwater	Depth below surface 0-0.025m	Depth below surface 0.025-0.5m	Total in highest risk categories				
		0%	0%	0%				



SHLAA / HELAA site reference		WB08180 and part of WB08045		
Site name		Grafton MSCP and part of Land at 51-93 Montague Street		
		The site has a low to medium risk of groundwater flooding, with groundwater le predicted to be between 0.5 and 5m below the surface during a 1% of groundwater flood event.		
		Tidal Groundwater Risk Zone (maximum risk)	Tidal Drainage Risk Zone (maximum risk)	
		GW2	SW3	
Tidal Risk Zone		The site is mostly located within Tidal Gro to most of the site being situated below the the site is also located within an area of m groundwater levels are more than 0.5m b groundwater flood event. The north and s within Tidal Groundwater Risk Zone GW1 situated at a higher elevation above the c tidal level and within an area of medium g groundwater levels are more than 0.5m b groundwater flood event.	bundwater Risk Zone GW2. This is due the present-day tidal level. Additionally, medium groundwater flood risk where elow the surface during a 1% AEP southern boundary of the site are located I. This is due to these areas being surrent tidal level but below the future groundwater flood risk, where elow the surface during a 1% AEP	
The centre of the site is partially located within Tid is due to this area being at risk during the 1% AE east, west and south of the site are located within This is due to these areas being located below th negligible risk from surface water flooding during The remainder of the site in the north is located SW1. This is due to these areas being located below the future tidal level, and at a negligible during the 1% AEP surface water event.			ithin Tidal Drainage Risk Zone SW3. This a 1% AEP surface water flood event. The ed within Tidal Drainage Risk Zone SW2. below the present-day tidal level but at a g during the 1% AEP surface water event. located within Tidal Drainage Risk Zone located above the current tidal level but egligible risk from surface water flooding	
	Reservoir	The site is not at risk of reservoir flooding.		



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	Defences	Defence T	уре	Standard of Protection		Condition
	Defences	There are no defen	ices within the	vicinit	y of the site.	
Flood risk management		Culvert / structure blockage?)	There are no known culverts or structures in the vicinity of the site.		
infrastructure	Residual risk	Impounded water body failure?		The site is not at risk of flooding due to reservoir breach.		
		Defence breach / overtopping?		The soverto	site is not at risk fror opping.	n defence breach or
	Flood warning		The majority of the site is located within the Environment Agency's 'Coastal areas of Rustington to Shoreham' (065WAC407) Flood Alert Area and the Environment Agency's 'Rustington, Worthing and Lancing' (065FWC2801) Flood Warning Area.			
Emergency planning	Access and egress	Dry access and egress could be available to the site to the north west via Augusta Place and Montague Street in all surface water events and present day coastal flood events.				
	Climate change allowances for the '2115 EPOCH' (2017 base year)	Proportion of site at 0.5% AEP coastal / tidal flood risk				
		Coastal region	Present d	ay	Higher Central	Upper End
		South East	n/a		+0.84m	+1.12m
			72%		100%	100%
Climate Change	Implications for the site	There is a significant increase in flood extent for both climate change allowances in comparison to the 0.5% AEP event. For the climate change scenarios, the flood extent reaches and exceeds that of the 0.1% AEP event, affecting the entire site. Therefore, climate change is predicted to have significant impact the proposed site.				
	Impact of climate	Propo	ortion of site	at 1% A	AEP surface water fl	ood risk
	change on risk from surface	Present day	+20% raii uplift	nfall	+30% rainfall uplift	+40% rainfall uplift
	water	5%	7%		8%	10%
	Implications for the site	A small increase in flood extent for the future 1% AEP surface water flood event predicted to occur for the plus 20%, 30% and 40% climate change event However, the extents do not reach that of the 0.1% AEP surface water flood ever These increases are located along the two flow routes from the east. Therefor the site will be at a higher risk from surface water flooding in the future.				e water flood event is nate change events. ace water flood event. the east. Therefore, the future.



SHLAA / HELAA site reference		WB08180 and part of WB08045
Site name		Grafton MSCP and part of Land at 51-93 Montague Street
	Bedrock Geology	The entire site's bedrock geology consists of Lewes Nodular Chalk Formation (chalk).
	Superficial Geology	The entire site is overlain with River Terrace Deposits (undifferentiated), sand, silt and clay.
	Soils	The site has freely draining slightly acid loamy soils.
	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.
	Historic Landfill Site	There are no historic landfill sites in the vicinity of the site.
Requirement for drainage control and impact mitigation		Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could provide wider sustainability benefits to the site and surrounding area. Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development. Most source control techniques are likely to be appropriate. Mapping suggests that permeable paving may have to use non-infiltrating systems given the possible medium risk from groundwater flooding. This must be confirmed via site investigations to assess the potential for infiltration. Whilst controlling run-off from proposed development must be addressed there is also a need to consider the
	Broad scale assessment of possible SuDS	flooding is not exacerbated at existing adjacent development. Infiltration techniques may be appropriate. Mapping suggests a medium risk of groundwater flooding and underlying soils may be permeable. Further site investigation must be carried out to assess potential for drainage by infiltration. If infiltration is suitable it should be avoided in areas where the depth to the water table is <1m. Given the high-density nature of the site, use of SuDS is recommended – urban sites should not preclude the use of SuDS.
		Detention techniques may be feasible, however mapping suggests mean site slopes could be greater than 5% in some areas, although this is generally due to the existing below ground parking on the site. Feasibility of such options must be assessed as part of a site-specific assessment. If this feature is feasible a liner maybe required due to the potential groundwater flooding on the site.
		Filtration options are unlikely to be feasible as mapping suggests mean site slopes are > 5%. Feasibility of such options must be assessed as part of a site-specific assessment, including an assessment of the seepage and storage capacity of the underlying soils. If this feature is feasible it must be located where the depth to the water table is >1m.
		All forms of conveyance are likely to be appropriate. Where the slopes are >5% features must follow contours or utilise check dams to slow flows. A liner maybe required to prevent the egress of groundwater.



Site name Grafton MSCP and part of Land at 51-93 Montague Street Cumulative impacts of development Water Framework Directive Catchment Sensitivity to cumulative impacts Coastal Catchment (not part of a river WB catchment) Medium Sequential Test and Exception Test requirements Medium The Sequential Test must be satisfied based on fluvial and other sources of flood risk before Exception test is applied. The Exception test will be required in the following scenarios:	SHLAA / HELA	ELAA site reference	WB08180 and part of WB08045			
Cumulative impacts of development Water Framework Directive Catchment Sensitivity to cumulative impacts Coastal Catchment (not part of a river WB catchment) Medium Sequential Test and Exception Test requirements Medium The Sequential Test must be satisfied based on fluvial and other sources of flood risk before Exception test is applied. The Exception test will be required in the following scenarios:	Site name		Grafton MSCP and part of Land at 51-93 Montague Street			
development Coastal Catchment (not part of a river WB catchment) Medium Sequential Test and Exception Test requirements The Sequential Test must be satisfied based on fluvial and other sources of flood risk before Exception test is applied. The Exception test will be required in the following scenarios:		Cumulative impacts of	Water Framework Directive Catchment	Sensitivity to cumulative impacts		
Sequential Test and Exception Test requirements The Sequential Test must be satisfied based on fluvial and other sources of flood risk before Exception test is applied. The Exception test will be required in the following scenarios:		development	Coastal Catchment (not part of a river WB catchment)	Medium		
The Sequential Test must be satisfied based on fluvial and other sources of flood risk before Exception test is applied.		Sequential Test an	d Exception Test requirements			
 If Highly vulnerable development is proposed to be located in FZ2. If More vulnerable or Essential Infrastructure development is proposed to be located in FZ3. If Essential infrastructure is proposed to be located in FZ3b. Development will not be permitted in the following scenarios: Highly vulnerable development within FZ3a. Highly vulnerable, More vulnerable and / or Less vulnerable development within FZ3b. 		The Sequential Test Exception test is ap The Exception test v If Highly vu If More vul If Essential Development will no Highly vulr Highly vulr	 The Sequential Test must be satisfied based on fluvial and other sources of flood risk before the Exception test is applied. The Exception test will be required in the following scenarios: If Highly vulnerable development is proposed to be located in FZ2. If More vulnerable or Essential Infrastructure development is proposed to be located in FZ3. If Essential infrastructure is proposed to be located in FZ3b. Development will not be permitted in the following scenarios: Highly vulnerable development within FZ3a. 			
Recommendations for requirements of site-specific Flood Risk Assessment, including guidar for developers		Recommendations for developers	for requirements of site-specific Flood Risk Assessment	, including guidance		
 Flood risk assessment: At the planning application stage, a site-specific flood risk assessment will be required for the site as development will be located within Flood Zone 3. It will also be required where development: 	Recommend- ations for Local Plan policy	Flood risk assess At the plan site as dev developme o m a o is dr o is dr o is dr o is o ris o o is o o is o o is o o is o o is o o is o o o is o o o is o o o is o o o o o o o o o o o o o o o o o o o	nent: ning application stage, a site-specific flood risk assessment will elopment will be located within Flood Zone 3. It will also be re- nt: ay be subject to other sources of flooding, where the developr more vulnerable use; on land which has been identified by the Environment Agency ainage problems; or on land identified in the strategic flood risk assessment as bei- ik in future. ces of flooding must be considered as part of any site-specific it, including surface water and groundwater. ion should be given to the potential effects of climate change, ter and coastal/ tidal flooding. Proposals should consider the hat provide for a reduction in the predicted flood risk at existin ange modelling should be undertaken using the relevant allow nt and level of risk. e is a reasonable likelihood of multiple sources of flood risk ha ombination it is recommended that consideration is given to as isks of these. ific risk assessment must demonstrate that surface water will velopment. n with the Local Authority, Lead Local Flood Authority and undertaken at an early stage. will need to demonstrate that users will be safe and more vulue 1 in 20 year flood extent where possible. lesign and making development safe: opment must seek opportunities to reduce the overall level of le, by: educing volume and rate of runoff	Ill be required for this equired where ment would introduce as having critical ing at increased flood flood risk with respect to opportunity to include g development. wances for the type of aving significant ssessing the not be displaced as a Environment Agency merable use is located f flood risk at the site.		



SHLAA / HELAA site reference		WB08180 and part of WB08045
Site name		Grafton MSCP and part of Land at 51-93 Montague Street
	 Safe access event and given to pro- All develop low impact More vulnes site. SuDS sho amenity, gr Example for reuse and Assessment Efforts sho not increass SuDS desis Statutory T 2015). Green infra runoff from space. Further det webpage surface wa LPA's required 	as and egress should be demonstrated in the fluvial 0.5% AEP plus climate change as there is a risk of surface water flooding on the site, consideration should also be oviding safe access and egress during surface water flood events. oment should adopt source control SuDS techniques to reduce the risk of frequent flooding due to post development runoff. arable land use is likely not be acceptable at ground floor level for the majority of the uld be designed to deliver multiple benefits including water quality, biodiversity, reen infrastructure etc. eatures include swales, attenuation features, green roofs, rainwater capture and permeable paving. Int of runoff should include allowances for climate change effects. uld be made to limit runoff to greenfield rates and discharge rates from the site should be downstream flood risk. gn must follow West Sussex County Council policy, meet the Defra National Non- Fechnical Standards, and follow current best design practice (CIRIA C753Manual astructure should be considered within the mitigation measures for surface water in potential development and consider using Flood Zones 2 and 3 as public open ails regarding Adur and Worthing Council requirements are available on the following <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms</u> . A ter drainage checklist is also available on this webpage. This clearly sets out the irements for avoiding pre-commencement conditions, or to discharge conditions.

Site name	Grafton MSCP and part of Land at 51-93 Montague Street	Adur and Worthing Level 2	ADUR & WORTHING	JBA	
Site area (ha)	0.77	Strategic Flood Risk Assessment Site Summary Sheet mapping	COUNCILS	consulting	
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Site name	Grafton MSCP and part of Land at 51-93 Montague Street	Adur and Worthing Level 2	ADUR & WORTHING	JBA
Site area (ha)	0.77	Strategic Flood Risk Assessment Site Summary Sheet mapping	COUNCILS	consulting

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SHLAA / HELAA site reference		WB08059
Site name		Land Site Decoy Farm, Dominion Way
	OS Grid reference	TQ 16034 04078
	Local Authority	Worthing Borough Council
	Area	7.36 ha
	Current land use	Former landfill/scrub
	Proposed site use	Minimum of 28,000m ² commercial
	Flood risk vulnerability	Less vulnerable
Site details	Topography	Site topography Legend is ite Boundary Image: Site Boundary Imag



SHLAA / HELAA	A site reference	WB08059				
Site name		Land Site Decoy Farm, Dominion Way				
	Existing watercourses	Since re-alignment work v the eastern site boundary which flows from north to section of watercourse is Dominion Way and Willow number of other locations	Since re-alignment work was completed in 2019 the Teville Stream flows along the eastern site boundary, from north to south. There is also a secondary channel which flows from north to south along the western and southern boundary. This section of watercourse is culverted between Deacon Way and the junction of Dominion Way and Willowbrook Road. This channel is also culverted at a number of other locations where there are access structures to the site.			
	Flood history	There are no historic flood	There are no historic flood records within the vicinity of the site.			
			Proportion of the site at risk	(
		(proportion reported a	re for the area of land occupie	ed by each flood extent		
		Percentages round	ed to the nearest 1%. Areas	< 0.5% not recorded)		
		5% AEP	1% AEP	0.1% AEP		
		13%	0%	3%		
Sources of flood risk	Fluvial	The site is covered by the Teville Stream (Fluvial) 2012 Flood Modeller-TUFLOW model. The extent of the Flood Zones predicted by the flood model are also the extent of the actual flood risk, as there are no flood risk management features that change the risk. It should be noted that this model contains a surface water component, as such, the depth, velocity and hazard mapping outputs shown include the surface water element as well as the fluvial risk. Re-alignment of the watercourse from the western and southern boundary to the eastern boundary was completed in 2010. This is not include in the ovisiting medalling and so the flood				
		 risk may differ slightly from Flood characteristics: The site is predicted to be Stream. A moderate section boundaries is to 13%). There is a further located within the section of the se	n that which is reported. at risk from fluvial flooding du tion of the site along the n cated within the 5% AEP flo r 3% of the site along the nort e 0.1% AEP flood extent.	e to the proximity of Teville orth, east and south site cod extent (approximately h and east site boundaries		
		Proportion of site at risk (RoFSW) (proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)				
		3.3% AEP	1% AEP	0.1% AEP		
	Surface Water	2% Description of surface w During the 3.33% AEP rai the site boundary in the no is a 5% increase in flood event. In the 0.1% AEP of all edges of the site, with the RoFSW considers flood ri	5% vater flow paths: infall event, the site is at a ver orth west, east and south, affe extent, predominantly in the event this flood extent increas the exception of a small section sk where the hazard rating is g	y low risk of flooding along cting 2% of the site. There north, during the 1% AEP ses a further 10% covering in in the south east. greater than 0.575.		

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SHLAA / HELA	A site reference	WB08059			
Site name		Land Site Decoy Farm, Dominion Way			
		Proportion of site at ris	k in JBA Grou	ndwater Ma	ap 1% AEP risk categories
		Depth below surface 0-0.025m	Depth below 0.025-0	/ surface).5m	Total in highest risk categories
	One standard and	12%	1%		13%
	Groundwater	The northern most section of the site (12%) has a high risk of groundwater flooding with groundwater levels predicted to be within 0.025m from the ground surface, during a 1% AEP groundwater flood event. Localised areas in the north (1%) have a medium risk of groundwater flooding, with levels predicted between 0.025 and 5m below the surface. The remainder of the site is it a negligible risk of groundwater flooding during the 1% AEP event.			
		Tidal Groundwater Risk Zone Tida (maximum risk)		al Drainage Risk Zone (maximum risk)	
	Tidal Risk Zones	GW4			SW4
		A small section in the north of the site is located within Tidal Groundwater Risk Zone GW4. This is due to this area being located below the present-day tidal level and in a high groundwater risk area, where groundwater levels are within 0.025m of the surface during a 1% AEP groundwater flood event. Small localised areas in the north are also located within Tidal Groundwater Risk Zone GW3. This is due to these areas being situated below present tidal level, with groundwater levels between 0.025 and 0.5m below the surface during a 1% AEP groundwater event. The rest of the site lies within Tidal Groundwater Risk Zone GW0 due to its location within an impermeable geological unit and therefore not at risk of being tidally influenced.			
A very small section along the northern boundary of the site is the Drainage Risk Zone SW4, where ground levels are below the prese and the area is at risk of flooding during the 1% AEP surface. The southern and eastern site boundaries are located within Ti Zone SW3, at risk of flooding from surface water flooding in the the present-day tidal level. Towards the centre of the site risk of Zones SW2 and SW1 as ground levels increase and risk of flooding during the site is located within Tidal D SW0 due to this area being located above the future tidal level risk of flooding during the 1% AEP surface water event.					the site is located within Tidal low the present-day tidal level P surface water flood event. If within Tidal Drainage Risk boding in the future and below e site risk decreases through I risk of flooding from surface hin Tidal Drainage Risk Zone tidal level and at a negligible nt.
	Reservoir	The site is not at risk of re	servoir flooding		

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Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT



SHLAA / HELA	HLAA / HELAA site reference WB08059				
Site name		Land Site Decoy Farm, Dominion Way			
	Cumulative impacts of	Water Framework Directive Catchment	Sensitivity to cumulative impacts		
	development	Teville Stream	Low		
	Sequential Test an	d Exception Test requirements			
	The Sequential Test Exception test is ap	st must be satisfied based on fluvial and other sources of plied.	flood risk before the		
	The Exception test v	vill be required in the following scenarios:			
	If Highly vuIf Essential	Inerable development is proposed to be located in FZ2. infrastructure is proposed to be located in FZ3b.			
	Development will no	t be permitted in the following scenarios:			
	Highly vuln Recommendations	erable, More vulnerable and / or Less vulnerable development	t within FZ3b		
	for developers	Tor requirements of site-specific Floor Kisk Assessment	, including guidance		
	Flood risk assessm	nent:			
	 At the planning application stage, a site-specific flood risk assessment will be required for this site as the site area is greater than one hectare. It will also be required where development is: 				
	 on land which may be subject to other sources of flooding, where the development would introduce a more vulnerable user 				
	 on land which has been identified by the Environment Agency as having critical 				
	dr	rainage problems; or n land identified in the strategic flood risk assessment as being at increased flood			
	risk in future.				
Recommend- ations for	Other sour	ces of flooding must be considered as part of any site-specific	flood risk		
Local Plan	assessment, including surface water and groundwater.				
poncy	respect to	surface water.			
	 Proposals should consider the opportunity to include measures that provide for a reduction in the predicted surface water flood risk at existing development. In particular, the scope should be explored for 'daylighting' of the culverted sections of the Teville Stream that run through the aite 				
	 Site. Climate chi developme 	ange modelling should be undertaken using the relevant allow nt and level of risk.	ances for the type of		
	Where there is a reasonable likelihood of multiple sources of flood risk having significant impact in combination it is recommended that consideration is given to assessing the combined risks of these.				
	Site specifi recent chail	c hydraulic modelling will need to be undertaken due to the ag	e of the model and		
	 Consultation with the Local Authority, Lead Local Flood Authority and Environment Agency should be undertaken at an early stage 				
	 Proposals will need to demonstrate that users will be safe and more vulnerable use is located outside Flood Zone 3b. 				
	Guidance for site o	lesign and making development safe:			
	 New development For examp R 	opment must seek opportunities to reduce the overall level of le, by: educing volume and rate of runoff	flood risk at the site.		
		elocating development to zones with lower flood risk			



SHLAA / HELAA site reference	WB08059
Site name	Land Site Decoy Farm, Dominion Way
Orfe annu	
 Sale acces event and a given to pro All develop low impact SuDS sho amenity, gr Example for reuse and The potent must be co Assessmer Efforts sho not increas SuDS desi Statutory T 2015). Green infra runoff from space. All existing divert, alter West Suss Further det webpage surface wa LPA's required 	as the egress should be demonstrated in the invital 1% AEP plus climate change as there is a risk of surface water flooding on the site, consideration should also be oviding safe access and egress during surface water flood events. ment should adopt source control SuDS techniques to reduce the risk of frequent flooding due to post development runoff. uld be designed to deliver multiple benefits including water quality, biodiversity, reen infrastructure etc. eatures include swales, attenuation features, green roofs, rainwater capture and permeable paving. tial impact of the culverted watercourses flowing to the east and south of the site, onsidered when designing site drainage and attenuation. Int of runoff should include allowances for climate change effects. Uld be made to limit runoff to greenfield rates and discharge rates from the site should e downstream flood risk. Ign must follow West Sussex County Council policy, meet the Defra National Non- rechnical Standards, and follow current best design practice (CIRIA C753 Manual astructure should be considered within the mitigation measures for surface water in potential development and consider using Flood Zones 2 and 3 as public open is watercourses should be retained and buffers to these provided. Any proposals to r, culvert infill or discharge to ordinary watercourses will require the prior consent of ex County Council as the Lead Local Flood Authority. ails regarding Adur and Worthing Council requirements are available on the following <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms</u> . A ter drainage checklist is also available on this webpage. This clearly sets out the irements for avoiding pre-commencement conditions, or to discharge conditions.



ROAL

60

120

Metres

60

120

Metres

VBROOK ROAD

at or very near (within 0.025m of) the ground surface.

120

☐ Metres

60

Site name	Land Site Decoy Farm, Dominion Way	Adur and Worthing Level 2	ADUR & WORTHING	JBA	
Site area (ha)	7.36	Strategic Flood Risk Assessment Site Summary Sheet mapping	COUNCILS	consulting	
All mans: Contains Ordnance Survey data @ Crown convright and database right 2020. Contains public sector information licensed under the Open Government License v3.0. Tidal Groundwater Risk Zones Man: Contains, IBA data @ IBA Consulting, 2020. Some of the responses contained in this manning are based					

All maps: Contains Ordnance Survey data © Crown copyright and database right 2020. Contains public sector information licensed under the Open Government License v3.0. Tidal Groundwater Risk Zones Map: Contains JBA data © JBA Consulting. 2020. Some of the responses contained in this mapping are based on data and information provided by the Natural Environment Research Council (NERC) or its component body the British Geological Survey (BGS). Your use of any information contained in this mapping is at your own risk. Neither JBA, NERC or BGS give any warranty, condition or representation as to the quality, accuracy or completeness of such information and all liability (including for negligence) arising from its use is excluded to the fullest extent permitted by law. Your use of the mapping constitutes your agreement to bring no claim against JBA, NERC or BGS in connection with it.





SHLAA / HELAA site reference		WB08183		
Site name		Land Site West of Fulbeck Avenue		
	OS Grid reference	TQ 10699 04735		
	Local Authority	Northing Borough Council		
	Area	2.00 ha		
	Current land use	Greenfield - unmanaged scrub and woodland		
	Proposed site use	50 residential units		
	Flood risk vulnerability	More vulnerable		
Site details	Topography	<figure><figure></figure></figure>		



SHLAA / HELA	A site reference	WB08183				
Site name		Land Site West of Fulbeck Avenue				
		1				
	Existing watercourses	Somerset's Lake is situate Barleyfields Stream flows and joins the Ferring Rife There are also two ordina Somerset's Lake embanki watercourse also receives and to the south the water which flows to the north of Finally, there is another sit to west north of the Barley	Somerset's Lake is situated 10m to the north west of the site, from which Barleyfields Stream flows north west to south east through the centre of the site and joins the Ferring Rife watercourse to the south. There are also two ordinary watercourses on the site which drain the toe of the Somerset's Lake embankment. To the north of Barleyfields Stream the watercourse also receives pumped flows from the Malthouse Way balancing pond and to the south the watercourse is a continuation of the Titnore Lane stream which flows to the north of the West Worthing Tennis and Squash Club. Finally, there is another small watercourse which flows through the site from east to west porth of the Barleyfields Stream			
	Flood history	There are no recorded flo	od events within the site.			
			Proportion of the site at risk	(
Sources of flood risk		(proportion reported a between larger or smal Percentages round	re for the area of land occupie ler return period events, and the ed to the nearest 1%. Areas <	ed by each flood extent herefore not cumulative. <0.5% not recorded)		
		5% AEP	1% AEP	0.1% AEP		
		5%	20%	6%		
	Fluvial	 This site is covered by the Environment Agency Ferring Rife (Fluvial/Tidal) 2019/20 Flood Modeller-TUFLOW model. The model was updated by JBA Consulting for Adur and Worthing Councils for the purpose of this SFRA. The extent of the Flood Zones predicted by the flood model are also the extent of the actual flood risk, as there are no flood risk management features that change the risk. Flood characteristics: The site is predicted to be at risk from fluvial flooding due to the proximity of Ferring Rife to the east of the site. A small section of the site in the north, and centre along the channel of the watercourse is located within Flood Zone 3b (approximately 5%) A further 20% (in the north of the site) is located within Flood Zone 3a. Finally, a further 6% of the site is located within Flood Zone 2 covering more central areas 				
		Proportion of site at risk (RoFSW)				
		(proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)				
		3.3% AEP	1% AEP	0.1% AEP		
		25%	5%	23%		
	Surface Water	Description of surface water flow paths:During the 3.3% AEP rainfall event, areas across the north east, and centre of the site are at risk of surface water flooding, as well as the southern boundary. There is a 5% increase in flood extent in the north east, centre and along the south eastern boundary during the 1% AEP event. For the 0.1% AEP event there is a further 23% increase in flood extent, with over half the site at risk, with the exception of the south west section.RoFSW takes account of building footprints so the flood risk may be affected by				
		existing buildings on the site. It also only considers flood risk where the hazard rating is greater than 0.575.				



SHLAA / HELAA site reference	WB08183
Site name	Land Site West of Fulbeck Avenue

		Proportion of site at risk in JBA Groundwater Map 1% AEP risk categories			
Groundwater	Depth below surface 0-0.025m	Depth below surface 0.025-0.5m		Total in highest risk categories	
	Groundwater	36%	0%)	36%
		Approximately a third of the site (36%) has a high risk of groundwater flooding with groundwater levels predicted to be less than 0.025m from the ground surface, during a 1% AEP groundwater flood event. This area is located to the south of the site. The remainder of the site is at a negligible risk of groundwater flooding.			
		Tidal Groundwater Risk Zone Ti (maximum risk)		Tida	l Drainage Risk Zone (maximum risk)
	Tidal Risk Zones	GW0			SW0
		The site is entirely located within Tidal Groundwater Risk Zone GW0 and Tidal Drainage Risk Zone SW0. This is due to the site being located above the future tidal level.			
	Reservoir	While the site is not at risk of flooding from reservoirs, there is a potential risk breach from Somerset's Lake or overtopping of the Malthouse Way balancing por (see section below for information on risk from impounded water bodies).			s, there is a potential risk of lthouse Way balancing pond nded water bodies).

WB08183

SHLAA / HELAA site reference

JBA consulting

Site name		Land Site West of Fulbeck Avenue				
			Defence Type	Standard	of Protection	Condition
		Defences	There are no defences within the vicinity of the site.			
			Culvert / structure blockage?	There is a culvert located to the south of the site which runs under Fulbeck Avenue. This culvert may pose a residual risk to the site in the event of a blockage.		
Flood risk managemen infrastructur	Flood risk management infrastructure	sk ent sure Residual risk	Impounded water body failure?	Somerset's Lake breach	Somerset's Lake poses a residual risk to the site in the event of a breach from the pond. Modelling has been undertaken to assess two different breach scenario locations, one at the outlet and one further to the north. Results of this modelling show that for a dry day, a breach in the north of the lake would cause flooding of 38% of the site across the north and centre. There is a 1% decrease in this flood extent in the north of the site for a southern breach. Maximum flood depths on the site are estimated to be 1.2m for a northern breach and 1.4m for a southern breach. For a wet day (0.1% AEP), a breach of this lake would significantly increase the risk of flooding to the site during the 0.1% AEP event. For a northern breach there would be a 20% increase in flood extent in the north compared to the dry day scenario. For a southern breach there is a 27% increase in flood extent compared to the day scenario. Maximum flood depths on site for the wet day scenario range between 1.2m for a northern breach and 1.6m for a southern breach.	
				Malthouse Way balancing pond overtopping	The northern section of the site (44%) is at risk of overtopping from the balancing pond on Malthouse Way during a 0.1% AEP event. Maximum flood depths of 0.6m are estimated in topographic lows.	
				Combination of both failures	The overtopping of and a breach of Sc a 0.1% event wou increase in risk to th in flood extent combination of the t to the northern brea is a 3% increase i combined failures southern breach of increases occur boundary of the site	f the balancing pond merset's Lake during Id result in a further le site. A 2% increase would occur for a wo failures compared ch only extent. There n flood extent of the compared to the ponly extent. These along the northern a.

Defence breach /

overtopping?

The site is not at risk from defence breach or overtopping.

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Adur and Worthing Councils

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT

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SHLAA / HELAA site reference		WB08183		
Site name		Land Site West of Fulbeck Avenue		
	Bedrock Geology	The majority of the site's bedrock geology is Lambeth Group- Clay, Silt and Sa The south west section of the site is formed of London Clay Formation- Clay, S and Sand.		
	Superficial Geology	The majority of the site's superficial geology is Head- Clay, S The south east section of the site is formed of River Terrace I (undifferentiated).	ilt, Sand and Gravel. Deposits	
	Soils	The centre of the site has freely draining slightly acid loamy s south of the site has slowly permeable seasonally wey slightly loamy and clayey soils.	oils. The north and y acid but base-rich	
Requirement for drainage control and impact mitigation	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.		
	Historic Landfill Site	There are no historic landfill sites in close proximity to the site.		
	Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could provide wider sustainability benefits to the site and surrounding area.		
		Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development.		
		Most source control techniques are likely to be appropriate. Mapping suggests that permeable paving may have to use non-infiltrating systems given the possible high risk from groundwater flooding in the south of the site. This must be confirmed via site investigations to assess the potential for infiltration. Whilst controlling run-off from proposed development must be addressed there is also a need to consider the effect of proposals on surface water flows such that predicted surface water flooding is not exacerbated at existing adjacent development.		
		Infiltration techniques may be appropriate. Mapping suggests a high risk of groundwater flooding in the south of the site and underlying soils may be permeable. Further site investigation must be carried out to assess potential for drainage by infiltration. If infiltration is suitable it should be avoided in areas where the depth to the water table is <1m.		
		Mapping suggests that the slope of the site makes it possible to consider most forms of detention. A liner maybe required due to the potential for groundwater flooding on the site.		
		Where there is not a significant risk of groundwater flooding, all infiltration techniques are likely to be appropriate, subject to confirming that the underlying soils have appropriate seepage and storage capacity.		
		All forms of conveyance are likely to be appropriate. Where the slopes are >5% features should follow contours or utilise check dams to slow flows. A liner maybe required to prevent the ingress of groundwater.		
	Cumulative impacts of development	Water Framework Directive Catchment	Sensitivity to cumulative impacts	
		Ferring Rife	Medium	

Adur and Worthing Councils

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT





SHLAA / HELAA site reference	WB08183			
Site name	Land Site West of Fulbeck Avenue			
 A greenfiel scheme to infrastructu Example for reuse and The potent must be co Assessmen from the sii SuDS desi Statutory T 2015). Green infra runoff from space. The residu that propo arrangeme structures a All existing divert, alten West Suss Further det webpage surface wa LPA's requ 	d site such as this should be able to implement an exemplar surface water drainage of deliver multiple benefits including water quality, biodiversity, amenity, green irre etc. eatures include swales, attenuation features, green roofs, rainwater capture and permeable paving. ial impact of the culvert which flows under Fulbeck Avenue, to the south of the site, insidered when designing site drainage and attenuation. In of runoff should include allowances for climate change effects, and discharge rates the should not increase downstream flood risk. Ign must follow West Sussex County Council policy, meet the Defra National Non- Technical Standards, and follow current best design practice (CIRIA C753 Manual astructure should be considered within the mitigation measures for surface water in potential development and consider using Flood Zones 2 and 3 as public open al risks associated with failure of the water retaining features must be addressed so ised development is safe. The considerations should include the appropriate ints and responsibilities for the maintenance and operation of water retaining as this will be directly linked to the likelihood of failure. I watercourses should be retained and buffers to these provided. Any proposals to r, culvert infill or discharge to ordinary watercourses will require the prior consent of ex County Council as the Lead Local Flood Authority. tails regarding Adur and Worthing Council requirements are available on the following https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms. A ter drainage checklist is also available on this webpage. This clearly sets out the irrements for avoiding pre-commencement conditions, or to discharge conditions.			



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Sitename	Land Site West of Fulbeck Avenue	Adur and Worthing Level 2	ADUR & WORTHING
Site area (ha)	2	Site Summary Sheet mapping	
All maps: Contains Ordnance on data and information prov accuracy or completeness of	e Survey data © Crown copyright and database right 2020. Contains public se rided by the Natural Environment Research Council (NERC) or its component f such information and all liability (including for negligence) arising from its use	ctor information licensed under the Open Government License v3.0. Tidal Groundv body the British Geological Survey (BGS). Your use of any information contained ir is excluded to the fullest extent permitted by law. Your use of the mapping constitu	water Risk Zones Map: Contains JBA data © JBA Consulting. 2020. Some of the responses contained in this mapping are based in this mapping is at your own risk. Neither JBA, NERC or BGS give any warranty, condition or representation as to the quality, sutes your agreement to bring no claim against JBA, NERC or BGS in connection with it.
Risk of Flood	ling from Surface Water- 3.33%,1% and 0.1%	Risk of Flooding from Surface Water (1% AEP	Depth) Risk of Flooding from Surface Water (1% AEP Velocity)
		Legend	











SHLAA / HELAA site reference		WB08163		
Site name		Land South of Upper Brighton Road		
	OS Grid reference	TQ 15618 04998		
	Local Authority	Worthing Borough Council		
	Area	5.83 ha		
	Current land use	Greenfield- arable fields and paddock		
	Proposed site use	123 residential units		
	Flood risk vulnerability	More vulnerable		
Site details	Topography	<complex-block> Site topography Legend Ite boundary Ite boundary</complex-block>		



SHLAA / HELA	A site reference	WB08163					
Site name Land South of Upper Brighton Road							
	Existing watercourses	Teville Stream (Main River) flows from north to south approximately 100m south east of the site. A drain cuts through the southern section of the site and flows for approximately 170m from the south west to north east site boundary. This drain forms part of the drainage network flowing into Teville Stream. There is also an ordinary watercourse which flows from north to south along the south eastern edge of the site.					
	Flood history	There are no historic flood	records within the vicinity of	of the site.			
		(proportion reported a between larger or small Percentages round	Proportion of the site at ri re for the area of land occup er return period events, and ed to the nearest 1%. Area	sk pied by each flood extent d therefore not cumulative. s <0.5% not recorded)			
		5% AEP	1% AEP	0.1% AEP			
Sources of flood risk	Fluvial	0% 0% 0% Available modelled data: The site is covered by the Teville Stream (Fluvial) 2012 Flood Modeller-TUFLOW model. The extent of the Flood Zones predicted by the flood model are also the extent of the actual flood risk, as there are no flood risk management features that change the risk. It should be noted that this model contains a surface water component, as such, the depth, velocity and hazard mapping outputs shown include the surface water element as well as the fluvial risk. Flood characteristics: The site is within Flood Zone 1 and is therefore at negligible risk of flooding form rivers. Proportion of site at risk (RoFSW) (proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative.					
		3.3% AEP	1% AEP	0.1% AEP			
		0%	1%	3%			
	Surface Water	Description of surface w The site is at a very low r event in two localised area is a small 3% increase in fl along a section of the wes RoFSW only considers flo	g during the 1% AEP rainfall here ponding occurs. There event in the south, north and ting is greater than 0.575.				
		Proportion of site at ris	k in JBA Groundwater Ma	p 1% AEP risk categories			
	Groundwater	Depth below surface 0-0.025m	Depth below surface 0.025-0.5m	Total in highest risk categories			
		35%	28%	63%			



SHLAA / HELAA site reference		WB08163		
Site name		Land South of Upper Brighton Road		
The south of the site, account with groundwater levels pred during a 1% AEP groundwate (28%) is located within an are the site, with groundwater lev the surface, during a 1% AEP site in the north has a lower r predicted to be at least 0.5m			h, has a high risk of groundwater flooding, within 0.025m from the ground surface ent. Approximately a quarter of the site m groundwater flood risk in the centre of ed to be between 0.025 and 0.5m below er flood event. The remaining 37% of the ndwater flooding, with groundwater levels urface.	
		Tidal Groundwater Risk Zone (maximum risk)	Tidal Drainage Risk Zone (maximum risk)	
		GW4	SW2	
	Tidal Risk Zones	Approximately three quarters of the site is located within Tidal Groundwater Risk Zone GW0, due to it being situated above future tidal level. The elevation of the site decreases to the south resulting in localised areas within tidal groundwater risk zones GW1,2,3 and 4. The southernmost section of the site is located within the maximum Tidal Groundwater Risk Zone GW4. This is due to this area being located within the vicinity of an area below the present-day tidal level and in a high-risk groundwater area where groundwater levels are within 0.025m of the surface during a 1% AEP groundwater flood event.		
		A very small section of the site along the southern boundary is located within Tidal Drainage Risk Zone SW2. This is due to this area being located above the present- day tidal level but at risk from surface water flooding in the future. The area immediately north of this section is located within Tidal Drainage Risk Zone SW1, above the current tidal level and at a negligible risk from surface water flooding. The remainder of the site is located above the future tidal level so is not at risk of being tidally influenced (SW0).		
	Reservoir The site is not at risk of reservoir flooding.			

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SHLAA / HELAA site reference		WB08163		
Site name		Land South of Upper Brighton Road		
	-			
	Bedrock Geology	The entire site's bedrock geology consists of Tarrant Chalk Member (chalk).		
	Superficial Geology	The entire site is overlain with Head (clay, silt, sand and gravel).		
	Soils	The site has freely draining slightly acid but base-rich soils.		
	Groundwater Source Protection Zone	Two thirds of the site in the south and west is located within Groundwater Source Protection Zone 1, which is defined as the 50 day travel time of pollutant to source with a 50m default minimum radius. The rest of the site in the north and east is located within Groundwater Protection Zone 2, which is defined as the 400 day travel time of pollutant to source. This has a 250 or 500m minimum radius around the source depending on the amount of water abstracted.		
	Historic Landfill Site	There are no historic landfill sites in the vicinity of the site.		
Requirement for drainage control and impact mitigation	Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could provide wider sustainability benefits to the site and surrounding area. Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development. Most source control techniques are likely to be appropriate. Mapping suggests that permeable paving may have to use non-infiltrating systems given the possible risk from groundwater (medium to high). This must be confirmed via site investigations to assess the potential for infiltration. Whilst controlling run-off from proposed development must be addressed there is also a need to consider the effect of proposals on surface water flows such that predicted surface water flooding is not exacerbated at existing adjacent development. The site is located within a Source Protection Zone. As such infiltration techniques must only be used where there are suitable levels of treatment although it is possible that infiltration may not be permitted in the south of the site, given the high risk of groundwater flooding. Proposed SuDS should be discussed with relevant stakeholders (LPA, LLFA and EA) at an early stage to understand possible constraints.		

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SHLAA / HELAA site reference		WB08163
Site name		Land South of Upper Brighton Road
	SuDS desi Statutory T	gn must follow West Sussex County Council policy, meet the Defra National Non-
	 2015). Green infra runoff from All existing divert, alter West Suss 	astructure should be considered within the mitigation measures for surface water potential development. watercourses should be retained and buffers to these provided. Any proposals to r, culvert infill or discharge to ordinary watercourses will require the prior consent of ex County Council as the Lead Local Flood Authority.
	 Further det following w A surface v LPA's requ 	ails regarding Adur and Worthing Council requirements are available on the ebpage <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms</u> . vater drainage checklist is also available on this webpage. This clearly sets out the irements for avoiding pre-commencement conditions, or to discharge conditions.









SHLAA / HELAA site reference		SB08329				
Site name		Local Green Space (LGS): Chatsmore Farm				
	OS Grid reference	TQ 10096 03424				
	Local Authority	Worthing Borough Council				
	Area	28.5 ha				
	Current land use	Greenfield- arable fields				
	Proposed site use	Maintain area as Local Green Space				
	Flood risk vulnerability	n/a				
Site details	Topography	Site topography Legend Site Boundary Elevation Image: Internet and the point of the p				



SHLAA / HELA	A site reference	SB08329				
Site name		Local Green Space (LGS): Chatsmore Farm				
		1				
	Existing watercourses	Ferring Rife (Main River) runs approximately 800m from east to west through the northern half of the site. The watercourse enters the site via two flow routes along the eastern boundary and flows to the western site boundary.				
	Flood history	The Environment Agency's Recorded Flood Outline dataset identifies a surface water flood incident in 1980 that impacted a small area in the north west corner of the site. Two further surface water incidents within the site were recorded in West Sussex County Council's dataset from the 2012 surface water and fluvial flood event. Both incidents are located along the Ferring Rife watercourse in the north east of the site, 130 and 140m from the eastern boundary, respectively.				
		Proportion of the site at risk				
		(proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)				
		5% AEP	1% AEP	0.1% AEP		
		3%	2%	5%		
Sources of flood risk	Fluvial	Available modelled data: This site is covered by the Environment Agency Ferring Rife (Fluvial/Tidal) 2019/20 Flood Modeller- TUFLOW model. The model was updated by JBA Consulting for Adur and Worthing Councils for the purpose of this SFRA. The extent of the Flood Zones predicted by the flood model is also the extent of the actual flood risk, as there are no flood risk management features that change the risk.				
		 Flood characteristics: The site is predicted to be at risk from fluvial flooding due to the Ferring Rife watercourse running through the north of the site. In the 5% AEP event flooding remains in bank and does not affect the site. Out of bank flooding on either side of the watercourse occurs for the 1% AEP event, resulting in the north west corner of the site being located within the 1% AEP flood extent (approximately 2%). A further 5% of the site in the north west corner is located within the 0.1% AEP flood extent. 				
		Pro	oportion of site at risk (RoFS	SW)		
	Surface Water	(proportion reported a between larger or smal Percentages round	re tor the area of land occupie ler return period events, and tl ed to the nearest 1%. Areas <	ed by each flood extent herefore not cumulative. <0.5% not recorded)		
		3.3% AEP	1% AEP	0.1% AEP		
		5%	4%	27%		



SHLAA / HELAA site reference		SB08329				
Site name		Local Green Space (LGS): Chatsmore Farm				
		Description of surface water flow paths: Flow paths largely remain within the Ferring Rife channel during the 3.3% AEP rainfall event, with a small area of ponding occurring south of the channel in the north west of the site. A further 4% of the site in the north west is impacted during the 1% AEP event. Flood extent is further exacerbated during the 0.1% AEP event where flow accumulation occurs along the entire southern reach of the Ferring Rife channel. RoFSW only considers flood risk where the hazard rating is greater than 0.575.				
		Proportion of site at risk in JBA Groundwater Map 1% AEP risk categories				
	Groundwater	Depth below surface 0-0.025m	Depth below surface 0.025-0.5m		Total in highest risk categories	
		90%	10	%	100%	
		The majority of the site has a high risk of groundwater flooding with groundwater levels predicted to be within 0.025m from the ground surface during a 1% AEP groundwater flood event. 10% of the site, including areas in the south and along the north and west site boundaries, have a medium risk of groundwater flooding, with levels predicted to be between 0.025 and 5m below the surface during a 1% AEP event.				
		Tidal Groundwater Risk Zone Tidal		Drainage Risk Zone		
		(maximum risk)		(maximum risk) SW3		
	Tidal Risk Zones	The site is mostly located within Tidal Groundwater Risk Zone GW0. Th to the site being situated above the future tidal level. Areas of greater ris (maximum Risk Zone GW3) are located along the reach of the Ferring R channel due to the lower topography but this risk remains within bank. Similarly, the majority of the site is located within Tidal Drainage Risk Z due to the site being situated above the future tidal level. Localised arr Ferring Rife channel are located at greater risk (maximum Risk Zone SW risk remains in bank and does not affect the site.				
	Reservoir	While the site is not at ris breach from Somerset's Po (see section below for info	k of flooding t ond or overtop rmation on ris	from reservoir pping of the Ma k from impou	rs, there is a potential risk of althouse Way balancing pond nded water bodies).	

SHLAA / HELAA site reference

JBA consulting

SHLAA / HELA	A site reference	SB08329				
Site name		Local Green Space (LGS): Chatsmore Farm				
		Type	Stand	ard of Protection	Condition	
Flood risk management infrastructure	Defences	High ground		2%	Good	
		Ferring Rife is reach that flow site for flood e	lined with high gro s through the site vents greater thar	ound on both sides of the cha . The defences do not provid n the standard of protection th	annel along the entire de protection to the nat they provide.	
		Culvert / structure blockage?		The Ferring Rife watercourse is culverted to the east and west of the site at Goring Street and Ferring Lane respectively. These culverts may pose a residual risk to the site in the event of a blockage		
	Residual risk	Impounded water body failure?	Somerset Lake breach	 blockage. Somerset Lake poses a residual risk to the sit in the event of a breach from the pond. Modellin has been undertaken to assess two different breach scenario locations, one at the outlet and one further to the north. Results of this modelling show that for a wet dat (0.1% AEP), a breach in the north of the lak would cause flooding of 13% of the site. Som flooding will remain within the Ferring Rift channel, with ponding occurring in the north were of the site. There is a 2% increase in this flood extent in the east of the site for a souther breach. Maximum out of channel flood depth on the site are estimated to be 0.9m for northern breach and 0.95m for a souther breach. The site is not at risk of flooding from a norther breach of this lake on a dry day. A very small residual risk exists in the east of the site for 		
			Malthouse Way balancing pond overtopping	The site is not at risk of flooding in the event the overtopping of the Malthouse Way balanci pond.		
			Combination of both failures	The overtopping of the b combination with a breac during a 0.1% AEP event w increase in risk to the si increase in flood extent for two failures compared to only extent. An increase ir occurs for the combined fai southern breach only ex increases occur in the east site. Maximum flood depth	palancing ponding in h of Somerset Lake ould result in a further ite. There is a 1% a combination of the the northern breach n flood extent of 0.5% lures compared to the ttent. These minor and north west of the is remain the same.	
		Defence bread overtopping?	ch /	The site is not at risk from overtopping.	n defence breach or	

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SHLAA / HELAA site reference		SB08329						
Site name		Local Green Space (LGS): Chatsmore Farm						
		•						
Emergency planning	Flood warning	The site is situated within the Environment Agency's 'North Ferring on the Ferring Rife' (065FWF5301) Flood Warning Area and the Environment Agency's 'Ferring Rife' (065WAF431) Flood Alert Area.						
	Access and egress	Dry access and egress could be available to the site to the north via the A259 in all surface water and fluvial/tidal events.						
		Pro	oportion of site	at 1% AEP fluv	ial flood	risk		
	Climate change allowances for '2080s'	River Basin District	Present day	Central	High Cent	ner tral	Upper End	
		South Fast	n/a	+35% flow uplift	+45% upl	flow ift	+105% flow uplift	
		Courr Euce	2%	8%	9%		13%	
Climate change	Implications for the site	A significant increase in flood extent is predicted for the climate change scena in comparison to the present day. The increases are located along the reach o Ferring Rife channel in the north west and east of the site. For the 1%AEP +1 CC scenario the flood extent reaches and exceeds that of 0.1% AEP fluvial ev Therefore, climate change is predicted to significantly impact the proposed site						
	Impact of climate	Proportion of site at 1% AEP surface water flood risk						
	change on risk from surface	Present day	+20% rainfal uplift	l +30% ra uplif	infall t	+4	0% rainfall uplift	
	water	9%	10%	10%	D		11%	
	Implications for the site	A slight increase in flood extent during the 1% AEP surface water flood event is predicted for the plus 20%, 30% and 40% climate change events. However, the extents do not reach that of the 0.1% AEP surface water flood extent. These increases are located within the north west and south of the site. Therefore, the site will be at a marginally higher risk from surface water flooding in the future.						



SHLAA / HELA	HLAA / HELAA site reference SB08329				
Site name		Local Green Space (LGS): Chatsmore Farm			
	Bedrock Geology	Two thirds of the site in the east and south is formed of Lewes Nodular Chalk Formation. A small area in the north west corner of the site is formed of Seaf Chalk Formation. The bedrock geology for the western section of the site is a mixture of Lewes Nodular Chalk Formation, Seaford Chalk Formation, Newha Chalk Formation, Culver Chalk Formation and Portsdown Chalk Formation (undifferentiated).			
	Superficial Geology	The Ferring Rife channel and banks are situated upon superficial deposits of Alluvium (clay, silt, sand and gravel). The remainder of the site has a superfic geology of River Terrace Deposits (undifferentiated).			
	Soils	The site has freely draining sightly acid loamy soils.			
	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.			
	Historic Landfill Site	There are no historic landfill sites in close proximity to the site.			
Requirement for drainage control and impact mitigation	Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportuni benefits including volume control, water quality, amenity an could provide wider sustainability benefits to the site and su Development at this site should not increase flood risk eithed design of the surface water management proposals should impacts of future climate change over the projected lifetime Most source control techniques are likely to be appropriate. that permeable paving may have to use non-infiltrating syst risk from groundwater flooding (medium to high across the confirmed via site investigations to assess the potential for controlling run-off from proposed development must be add need to consider the effect of proposals on surface water fle predicted surface water flooding is not exacerbated at exist development. Mapping suggests that there is a high risk of groundwater fle therefore it is possible infiltration and filtration techniques w This must be confirmed via site investigations to assess the infiltration by examining the seepage and storage capacity of Detention may be feasible provided site slopes are < 5% at detention feature. A liner maybe required due to the potent flooding on the site. All forms of conveyance are likely to be appropriate. Where	ties to deliver multiple d biodiversity. This irrounding area. er on or off site. The take into account the of the development. Mapping suggests ems given the possible site). This must be infiltration. Whilst iressed there is also a pws such that ing adjacent ooding at this location, ill not be appropriate. a potential for of the underlying soils. the location of the ial for groundwater		
		teatures should follow contours or utilise check dams to slo required to prevent the ingress of groundwater.	w flows. A liner maybe Sensitivity to		
	Cumulative		cumulative impacts		
	development	Ferring Rife	Medium		

Adur and Worthing Councils

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SHLAA / HELAA site reference		SB08329
Site name		Local Green Space (LGS): Chatsmore Farm
Site name Recommend- ations for Local Plan policy	Sequential Test an The Sequential Test an The Sequential Test Exception test is ap The Exception test w If Highly vu If More vulle If developring roundwate Development will no Highly vulne Highly vulne Highly vulne Highly vulne Recommendations for developers Flood risk assessme At the plan site as the o lo o or in o or dr o or cist Other sour assessmer Lake). Considerate respect to a to include r Climate ch developme Where there impact in c combined r Consultations should be r Proposals outside Flood New devel For examp o R	Local Green Space (LGS): Chatsmore Farm d Exception Test requirements t must be satisfied based on fluvial and other sources of flood risk before the plied. will be required in the following scenarios: unerable development is proposed to be located in FZ2. nerable or Essential Infrastructure development is proposed to be located in FZ3. infrastructure is proposed to be located in FZ3. ment is proposed within areas at high risk of other sources of flooding including er and surface water. t be permitted in the following scenarios: erable development within FZ3a. erable development within FZ3a. erable development within FZ3a. for requirements of site-specific Flood Risk Assessment, including guidance nent: ning application stage, a site-specific flood risk assessment will be required for this site area is greater than one hectare. It will also be required where development is: cated in Flood Zones 2 or 3: n land which is subject to other sources of flooding, where the development is: cated in Flood Zones 2 or 3: n land which has been identified by the Environment Agency as having critical ainage problems; or h land dichtified in the strategic flood risk assessment as being at increased flood sk in future. ces of flooding must be considered as part of any site-specific flood risk t, including surface water, groundwater and impounded waterbodies (Somerset's ion should be given to the potential effects of climate change, particularly with surface water and fluvial/tidal flooding. Proposals should consider the opportunity neasures that provide for a reduction in the flood risk at existing development. ange modelling should be undertaken using the relevant allowances for the type of nt and level of risk. re is a reasonable likelihood of multiple sources of flood risk having significant ombination it is recommended that consideration is given to assessing the isks of these. will need to demonstrate that users will be safe and more vulnerable use is located od Zone 30.
	Safe acces event and given to pro- All develop low impact	reating space for flooding. ss and egress should be demonstrated in the fluvial 1% AEP plus climate change as there is a risk of surface water flooding on the site, consideration should also be oviding safe access and egress during surface water flood events. oment should adopt source control SuDS techniques to reduce the risk of frequent flooding due to post development runoff.



SHLAA / HELAA site reference	SB08329
Site name	Local Green Space (LGS): Chatsmore Farm
 A greenfiel scheme to infrastructu Example for reuse and The potent the site, mu Assessmen from the sit SuDS desi Statutory T 2015). Green infra runoff from space. The residu that propo arrangeme structures a All existing divert, alter West Suss Further det webpage surface wa LPA's required 	Id site such as this should be able to implement an exemplar surface water drainage of deliver multiple benefits including water quality, biodiversity, amenity, green are etc. eatures include swales, attenuation features, green roofs, rainwater capture and permeable paving. tial impact of the culverts along the Ferring Rife watercourse to the east and west of ust be considered when designing site drainage and attenuation. In of runoff should include allowances for climate change effects, and discharge rates te should not increase downstream flood risk. tign must follow West Sussex County Council policy, meet the Defra National Non- Technical Standards, and follow current best design practice (CIRIA C753 Manual astructure should be considered within the mitigation measures for surface water in potential development and consider using Flood Zones 2 and 3 as public open and risks associated with failure of the water retaining features must be addressed so used development is safe. The considerations should include the appropriate ents and responsibilities for the maintenance and operation of water retaining as this will be directly linked to the likelihood of failure. If watercourses should be retained and buffers to these provided. Any proposals to r, culvert infill or discharge to ordinary watercourses will require the prior consent of exe County Council as the Lead Local Flood Authority. tails regarding Adur and Worthing Council requirements are available on the following <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms.</u> A ater drainage checklist is also available on this webpage. This clearly sets out the uirements for avoiding pre-commencement conditions, or to discharge conditions.







⊐Metres

⊐Metres

Metres

Site name	Local Green Space (LGS): Chatsmore Farm	Adur and Worthing Level 2 Strategic Flood Risk Assessmen	ADUR & WORTHING	JBA	
Site area (ha)	28.5	Site Summary Sheet mapping	COUNCILS	consulting	
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Site name	Local Green Space (LGS): Chatsmore Farm	Adur and Worthing Level 2	ADUR & WORTHING	JBA
Site area (ha)	28.5	Site Summary Sheet mapping	COUNCILS	consulting





SHLAA / HELAA site reference		HT08283
Site name		New Salts Farm Lancing
	OS Grid reference	TQ 20023 04735
	Local Authority	Adur District Council
	Area	29.24 ha
	Current land use	Greenfield
	Proposed site use	Residential development with narrow areas of open space
	Flood risk vulnerability	More vulnerable
Site details	Topography	<complex-block> Ste topography Image: Ste Boundary Image: Ste Boundary<!--</th--></complex-block>



SHLAA / HELAA site reference		HT08283			
Site name		New Salts Farm Lancing			
	Existing watercourses	A number of drainage ditches flow along the northern site boundary from west to east. Another drainage ditch intersects the site from the western boundary and runs through the site to the north east corner. There is also a "blind" ordinary watercourse along the southern boundary and a further ordinary watercourse running for a short length along the western boundary of the site. These drainage features ultimately drain to an outfall into the River Arun, downstream of the Dogs Trust rehoming centre near New Salts Farm Road (approximately 700m to the east of the site).			
	Flood history	The Environment Agency's Recorded Flood Outline dataset identifies that the west of the site flooded in 1960 and 1986. The cause of flooding for both events was reported to be linked to local drainage and surface water issues.			
		(proportion reported a between larger or small Percentages rounde	Proportion of the site at risk re for the area of land occupie er return period events, and the ed to the nearest 1%. Areas	¢ ed by each flood extent herefore not cumulative. <0.5% not recorded)	
		5% AEP	0.5% AEP	0.1% AEP	
		0%	0%	0%	
Sources of flood risk	Coastal / tidal	Available modelled data: The site is covered by the Environment Agency's River Adur (Fluvial/Tidal) 2018 Flood Modeller-TUFLOW model. The model was updated by JBA Consulting for Adur and Worthing Councils for the purpose of this SFRA. The extent of the Flood Zones predicted by the flood model are different to the extent of the actual flood risk, as there are flood risk management features that change the risk. Flood characteristics: The entire site is located within Flood Zone 3a and at risk of flooding from the River Adur to the east and the sea to the south. However, when flood risk management features are accounted for the site is at a negligible risk of coastal/tidal flooding during the 0.5% AEP event as it is protected by the defences along the River Adur and the beach to the south.			
		(proportion reported a	5W) ed by each flood extent		
		between larger or smaller return period events, and therefore not cumulative.			
		3.3% AEP	1% AEP	0.1% AEP	
		0%	1%	10%	
	Surface Water	 Description of surface water flow paths: The site is at a very low risk of surface water flooding (less than 0.5%) in the centre and south east corner during the 3.33% AEP rainfall event. There is a further 1% increase in flood extent for the 1% AEP event concentrated in the south east corner of the site. For the 0.1% AEP event, there is a 10% increase in flood extent in localised areas across the site. The largest area of increase is to the west of the site where a flow path develops in easterly direction across the site. RoFSW only considers flood risk where the hazard rating is greater than 0.575. It also does not take account of the impacts of tide locking on the drainage from the site. 			



SHLAA / HELAA site reference		HT08283			
Site name		New Salts Farm Lancing			
		Proportion of site at risk in JBA Groundwater Map 1% AEP risk categories			
		Depth below surface 0-0.025m	Depth below 0.025-0	.5m	Total in highest risk categories
		n/a	n/a		n/a
	Groundwater	As part of the Lancing Surface Water Management Plan a detailed assessment of the geology and hydrogeology was carried out for an area which includes this site. For the purpose of this assessment the information contained in the SWMP has been used rather than the national JBA Groundwater mapping as there are a number of localised features which affect groundwater levels and which are not captured in the national scale mapping. There are two distinct groundwater aquifers under the site, one in the lower chalk strata and the other within a layer of superficial deposits (mainly Alluvium) which overlay the chalk. These two aquifers are separated by layers of clay forming an "aquitard" which limits movement between the two aquifers. The Alluvium aquifer is largely recharged by rainfall and is drained by evapotranspiration and through lateral flow to surface water. However, there are locations where the separation between the aquifers is less marked due to the presence of more permeable "windows" between the Chalk and upper aquifers. Under conditions of high winter recharge there may be upward leakage from the Chalk to the upper aquifer and surface water through the more permeable "windows" in the Superficial Deposits. Finally, diurnal changes in the Chalk piezometric surface have been observed near the coast in response to the rise and fall of the tide level. As a result of these influences. The site is considered to be at high risk of groundwater flooding.			
		Tidal Groundwater R (maximum ris	isk Zone k)	Tida	l Drainage Risk Zone (maximum risk)
		GW4	- 1		SW3
	Tidal Risk Zones	The Tidal Ground Water Risk Zones have been calculated using the JBA national Groundwater Flood mapping and as a result the tidal groundwater risk is likely to be higher than indicated due to the unique local hydrogeology. The site is largely situated below the present-day tidal level and is at high risk of groundwater flooding and previous studies have shown there is a link between tide levels and the groundwater levels The site is mostly located in Tidal Drainage Risk Zone SW2. This is due to the site being below the present-day tidal level and mostly at a negligible risk from surface water flooding during the 1% AEP surface water event. Localised areas across the site are situated within Tidal Drainage Risk Zone SW3. These correlate to areas below the present-day tidal level and at risk from surface water flooding in the future.			
Reservoir		The site is not at risk of reservoir flooding.			

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Adur and Worthing Councils

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SHLAA / HELAA site reference		HT08283		
Site name		New Salts Farm Lancing		
	Bedrock Geology	The entire site's bedrock geology consists of White Chalk.		
	Superficial Geology	The majority of the site is overlain with alluvium deposits (clar small corner of the south east corner of the site does not con deposits.	y, silt and sand). A tain any superficial	
	Soils	The site predominantly has loamy and clayey soils of coastal high groundwater. A very small section in the south east of th dune soils.	flats with natural ne site has sand	
	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.		
	Historic Landfill Site	A historic landfill site is located 120m south east of the site.		
Requirement for drainage control and impact mitigation	Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportunities to deliver mu benefits including volume control, water quality, amenity and biodiversity. Th could provide wider sustainability benefits to the site and surrounding area. Development at this site should not increase flood risk either on or off site. T design of the surface water management proposals should take into account impacts of future climate change over the projected lifetime of the development Most source control techniques are likely to be appropriate. Background understanding of the groundwater conditions suggests that permeable pavin may have to use non-infiltrating systems across the site due to the high risk groundwater flooding. This must be confirmed via site investigations to asses the potential for infiltration. Whilst controlling run-off from proposed develop must be addressed there is also a need to consider the effect of proposals o surface water flows such that predicted surface water flooding is not exacert at existing adjacent development. It is possible infiltration and filtration techniques will not be appropriate. This be confirmed via site investigations to assess the potential for infiltration by examining the seepage and storage capacity of the underlying soils. Mapping suggests that the slope of the site makes it possible to consider mc forms of detention. A liner may be required due to the potential for groundwat flooding to the south east of the site. All forms of conveyance are likely to be appropriate. Where the slopes are > features should follow contours or utilise check dams to slow flows. A liner r required to prevent the ingress of groundwater.		
	Cumulative impacts of development	Water Framework Directive Catchment	Sensitivity to cumulative impacts	
		River Adur (not part of a river water basin catchment)	Medium	

Adur and Worthing Councils

Level 2 SFRA Detailed Site Summary Tables – DRAFT DOCUMENT

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SHLAA / HELAA site reference		HT08283
Site name		New Salts Farm Lancing
	· · · · ·	
	 SuDS desi Statutory T 2015). Green infra runoff from 	gn must follow West Sussex County Council policy, meet the Defra National Non- Technical Standards, and follow current best design practice (CIRIA C753 Manual astructure should be considered within the mitigation measures for surface water
	 All existing watercourses should be retained and buffers to these provided. Any proposal divert, alter, culvert infill or discharge to ordinary watercourses will require the prior conser West Sussex County Council as the Lead Local Flood Authority. 	
	Further det webpage surface wa LPA's requ	ails regarding Adur and Worthing Council requirements are available on the following <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms</u> . A ater drainage checklist is also available on this webpage. This clearly sets out the irrements for avoiding pre-commencement conditions, or to discharge conditions.

Site name New Salts Farm Site Lancing Adur and Worthing Level 2 Adur and Worthing Le

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Shite-name New Salts Farm Site Lancing Adur and Worthing Level 2 Adur and Worthing L

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SHLAA / HELAA site reference

OS Grid

Area

use

reference

Local Authority

Current land use

Proposed site

Flood risk

vulnerability

Topography

Site name

Site details



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- The site is generally flat with a slight slope from west to the north east of • the site.
- There a few small existing buildings located on the site but overall, the site is largely green field.
- The ground slope across the site generally has a gradient of less than 5%


SHLAA / HELAA site reference		HN08235				
Site name		Old Salts Farm Site, Lanc	ing			
	Existing watercourses	An ordinary watercourse flows through the site from the west boundary to the north east corner. Three smaller drainage ditches are also located in the centre and north and west corners of the site. These drainage features ultimately drain to an outfall into the River Arun, downstream of the Dogs Trust rehoming centre near New Salts Farm Road (approximately 1.5km to the east of the site).				
	Flood history	The Environment Agency's recorded flood outlines dataset displays two events occurring in 1960 and 1986. These events are reported to have occurred due to drainage and surface water issues. West Sussex County Council's recorded flood incidents dataset also recorded three incidents within 50m of the south of the site. Each incident is reported to have occurred during the 1974 flood event as a result of drainage issues.				
			Proportion of the site at risk	(
		(proportion reported a between larger or smal Percentages round	re for the area of land occupie ler return period events, and the ed to the nearest 1%. Areas	d by each flood extent nerefore not cumulative. <0.5% not recorded)		
		5% AEP	0.5% AEP	0.1% AEP		
		0%	0%	2%		
Sources of flood risk	Coastal / tidal	 Available modelled data. The site is covered by the Environment Agency's River Adur (Fluvial/Tidal) 2018 Flood Modeller - TUFLOW model. The model was updated by JBA Consulting for Adur and Worthing Councils for the purpose of this SFRA. The extent of the Flood Zones predicted by the flood model are different to the extent of the actual flood risk, as there are flood risk management features that change the risk. Flood characteristics: Almost the entire site is located within Flood Zone 3a and at risk of flooding from the River Adur to the east and the see to the south. However, when flood risk management features are accounted for the site has a negligible risk of flooding for the 5% and 0.5% AEP events as it is protected by the defences along the River Adur and the beach to the south. However, for the 0.1% AEP event, there is a small risk of flooding to the west of the site. 				
		(proportion reported are for the area of land occupied by each flood extent				
		between larger or smal	ler return period events, and the	nerefore not cumulative.		
				0.5% ΠΟΙ Tecorded)		
		Less than 1%	4%	13%		
		Description of surface w	vater flow paths:			
	Surface Water	The site is at a very low risk of surface water flooding (less than 1%) in the west during the 3.33% AEP rainfall event. There is a 4% increase in flood extent for the 1% AEP event involving small patches of accumulation across the site. A dominant flow route from west to east, across the centre of the site, is formed. For the 0.1% AEP event, there is further 13% increase in flood extent. A proportion of this relates to an additional flow route which forms from the north east corner of the site flowing south. There is also an expansion in the smaller patches of accumulation.				
		rating is greater than 0.575 and it does not take account of the impacts of tide locking on the drainage from the site.				

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SHLAA / HELAA site reference		HN08235				
Site name		Old Salts Farm Site, Lancing				
		Proportion of site at risk in JBA Groundwater Map 1% AEP risk categories Depth below surface 0-0.025m Depth below surface 0.025-0.5m Dotal in highest risk				
		n/a	n/a		n/a	
	Groundwater	As part of the Lancing Surface Water Management Plan a detailed assessment of the geology and hydrogeology was carried out for an area which includes this site. For the purpose of this assessment the information contained in the SWMP has been used rather than the national JBA Groundwater mapping as there are a number of localised features which affect groundwater levels and which are not captured in the national scale mapping. There are two distinct groundwater aquifers under the site, one in the lower chalk strata and the other within a layer of superficial deposits (mainly Alluvium) which overlay the chalk. These two aquifers are separated by layers of clay forming an "aquitard" which limits movement between the two aquifers. The Alluvium aquifer is largely recharged by rainfall and is drained by evapotranspiration and through lateral flow to surface water. However, there are locations where the separation between the aquifers is less marked due to the presence of more permeable "windows" between the Chalk and upper aquifers. Under conditions of high winter recharge there may be upward leakage from the Chalk to the upper aquifer and surface water through the more permeable "windows" in the Superficial Deposits. Finally, diurnal changes in the Chalk piezometric surface have been observed near the coast in response to the rise and fall of the tide level. As a result of these influences. The site is considered to be at high risk of groundwater flooding.				
		Tidal Groundwater R (maximum ris	lisk Zone k)	Tida	al Drainage Risk Zone (maximum risk)	
	Tidal Risk Zones	GW4			SW4	
		The Tidal Ground Water Risk Zones have been calculated using the JBA national Groundwater Flood mapping and as a result the tidal groundwater risk is likely to be higher than indicated due to the unique local hydrogeology. The site is largely situated below the present-day tidal level and is at high risk of groundwater flooding and previous studies have shown there is a link between tide levels and the groundwater levels.				
		The site is mostly located within Tidal Drainage Risk Zone SW2 due to the site being located below present-day tidal level and at a negligible risk of surface water flooding during the 1% AEP surface water event. Small sections of the site are located within Tidal Drainage Risk Zone SW3. These correlate to areas at risk from surface water flooding in the future. Two localised areas in the west of the site are located within Tidal Drainage Risk Zone SW4. This is due to their location below the present-day tidal level and within an area at risk during the 1% AEP surface water flood event.				
	Reservoir	The site is not at risk of flo	oding of reserv	oir flooding.		

SHLAA / HELAA site reference HN08235						
Site name		Old Salts Farm Site, Lancing				
		Defence 1	vpe	Stan	dard of Protection	Condition
Flood risk	Defences	There are no defences within the site. However, the Shoreham Tidal Walls a located approximately 1.25km east of the site and there is a raised defence t south of the site, adjacent to Brighton Road. However this defence has hous built on top of it and has been subject to numerous incidents where sections been removed during construction works.				
		Culvert / structure blockage?	9	There vicinit	There are no known culverts or structures in the vicinity of the site	
infrastructure		Impounded water failure?	body	The s breac	ite is not at risk of floo h.	oding due to reservoir
	Residual risk	Defence breach / overtopping?		The s infrast from c of the the en has th	site benefits from floo tructure, therefore the defence breach or ove undefended 1% AEF ntire site other than t ne potential to be at ris	od risk management e site could be at risk ertopping. The extent P event indicates that he north west corner sk during a breach.
Emergency	Flood warning	The site is located within the Environment Agency's 'Shoreham Town and Lancing' (065FWC3002) Flood Warning Area and the Environment Agency's 'Inland areas of Shoreham, Lancing and Southwick' (065WAC409) Flood Alert Area.				
planning	Access and egress	Dry access and eccoastal flood even	gress could be ts via Old Salts	availa s Farm	ble to the site during Road in the north we	all surface water and st corner of the site.
	Climate change allowances for the '2115 EPOCH' (2017 base year)	Proportion of site at 0.5% AEP coastal / tidal flood risk				
		Coastal region	Present d	lay	Higher Central	Upper End
		South Foot	n/a		+0.84m	+1.12m
		0%			70%	91%
Climate Change	Implications for the site	There is an increase in flood extent for both modelled climate change comparison to the 0.5% AEP event. For both climate change allowances extent reaches and exceeds that of the 0.1% AEP event, affecting almost site for the upper end. Therefore, climate change is predicted to ir proposed site. The increase in flood risk is due to the Shoreham tidal w overtopped by the increased sea levels.				ate change events in allowances the flood cting almost the entire dicted to impact the ham tidal walls being
	Impact of climate	Propo	ortion of site	at 1% A	AEP surface water fl	ood risk
	change on risk from surface	Present day	+20% raii uplift	nfall	+30% rainfall uplift	+40% rainfall uplift
	water	4%	7%		8%	10%
	Implications for the site	4% 1% Small increases in flood extent during the 1% AEP surface water flood event are predicted for the plus 20%, 30% and 40% climate change events. However, the extents do not reach that of the 0.1% AEP surface water flood event. These increases are located within the east and west of the site. Therefore, the site will be at a higher risk from surface water flooding in the future. However, it should be noted that this dataset does not take account of the impact of tide locking from increased sea levels on drainage from the site.				





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SHLAA / HELAA site reference		HN08235			
Site name		Old Salts Farm Site, Lancing			
		-			
	Bedrock Geology	The entire site's bedrock consists of White Chalk.			
	Superficial Geology	Almost the entire site is overlain with alluvium deposits (clay, small corner to the west of the site is overlain with sand and g	silt and sand). A gravel deposits.		
	Soils	The site has loamy and clayey soils of coastal flats with natur groundwater.	ally high		
	Groundwater Source Protection Zone	The site is not within a Groundwater Source Protection Zone.			
	Historic Landfill Site	There are no historic landfill sites located within close proxim	ity of the site.		
Requirement for drainage control and impact mitigation	Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportunitie benefits including volume control, water quality, amenity and could provide wider sustainability benefits to the site and surr Development at this site should not increase flood risk either design of the surface water management proposals should ta impacts of future climate change over the projected lifetime o Most source control techniques are likely to be appropriate. If understanding of the groundwater conditions suggests that pr may have to use non-infiltrating systems across the site due t groundwater flooding. This must be confirmed via site invest the potential for infiltration. Whilst controlling run-off from pro must be addressed there is also a need to consider the effect surface water flows such that predicted surface water flooding at existing adjacent development. It is possible infiltration and filtration techniques will not be ap be confirmed via site investigations to assess the potential fo examining the seepage and storage capacity of the underlyin Mapping suggests that the slope of the site makes it possible forms of detention. A liner may be required due to the potent flooding across the site All forms of conveyance are likely to be appropriate. Where the features should follow contours or utilise check dams to slow required to prevent the ingress of groundwater.	es to deliver multiple biodiversity. This ounding area. on or off site. The ike into account the f the development. Background ermeable paving to the high risk of igations to assess iposed development t of proposals on g is not exacerbated inpropriate. This must r infiltration by g soils. to consider most ial for groundwater the slopes are >5% flows. A liner maybe		
	Cumulative impacts of	Water Framework Directive Catchment	Sensitivity to cumulative impacts		
impacts of development		River Adur (not part of a river water basin catchment)			

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SHLAA / HELAA site reference		HN08235
Site name		Old Salts Farm Site, Lancing
	 Assessmer from the sit SuDS desi Statutory T 2015). Green infra runoff from 	nt of runoff should include allowances for climate change effects, and discharge rates the should not increase downstream flood risk. gn must follow West Sussex County Council policy, meet the Defra National Non- Technical Standards, and follow current best design practice (CIRIA C753 Manual astructure should be considered within the mitigation measures for surface water potential development.
	 All existing watercourses should be retained and buffers to these provided. Any proposals divert, alter, culvert infill or discharge to ordinary watercourses will require the prior consent West Sussex County Council as the Lead Local Flood Authority. Further details regarding Adur and Worthing Council requirements are available on the following webpage https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms. surface water drainage checklist is also available on this webpage. This clearly sets out a LPA's requirements for avoiding pre-commencement conditions, or to discharge conditions. 	

Site name Old Salts Farm Site, Lancing Adur and Worthing Level 2 Adur and Worthing L

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Site name Old Salts Farm Site, Lancing Adur and Worthing Level 2 Adur and Worthing L

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SHLAA / HELAA site reference		FB08229				
Site name		Shoreham Gateway Site				
	1	1				
	Existing watercourses	The River Adur (main river) is located 40m to the west of the site and flows from north to south.				
	Flood history	There are no historic flood incidents recorded at the site.				
			Proportion of the site at ri	sk		
		(proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)				
		5% AEP	0.5% AEP	0.1% AEP		
		0%	7%	39%		
	Coastal / tidal	Available modelled data The site is covered by the Flood Modeller – TUFLOV Adur and Worthing Counc Zones predicted by the flo risk, as there are flood risk	: e Environment Agency's Riv V model. The model was up ils for the purpose of this SF ood model are different to th k management features that	ver Adur (Fluvial/Tidal) 2018 odated by JBA Consulting for RA. The extent of the Flood ne extent of the actual flood change the risk.		
		Flood characteristics: The entire site is located within Flood Zone 3a and at risk of flooding from the River Adur to the east. However, when flood risk management features are accounted for the site has a reduced risk of flooding. The 0.5% AEP extent is predicted to flood along the west site boundary. This is further increased during the 0.1% AEP event which affects the north west corner of the site.				
Sources of		Proportion of site at risk (RoFSW)				
tiood risk		(proportion reported are for the area of land occupied by each flood extent between larger or smaller return period events, and therefore not cumulative. Percentages rounded to the nearest 1%. Areas <0.5% not recorded)				
		3.3% AEP	1% AEP	0.1% AEP		
		9%	11%	26%		
	Surface Water	Description of surface water flow paths: The site is at a moderate risk of surface water flooding along the western boundary during the 3.3% AEP rainfall event (9%). There is an 11% increase in flood extent during the 1% AEP with flood risk remaining in the west of the site. In the 0.1% AEP event there is a further 26% increase in flood extent, extending into the centre of the site from the west and affecting just under half the site. RoFSW only considers flood risk where the hazard rating is greater than 0.575.				
		Proportion of site at risk in JBA Groundwater Map 1% AEP risk categories				
		Depth below surface 0-0.025m	Depth below surface 0.025-0.5m	Total in highest risk categories		
	Groundwater	n/a	n/a	n/a		
		As part of the Lancing Sur the geology and hydrogeo For the purpose of this as been used rather than the number of localised featured captured in the national so	an a detailed assessment of area which includes this site. contained in the SWMP has ter mapping as there are a er levels and which are not			

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SHLAA / HELA	A site reference	FB08229		
Site name		Shoreham Gateway Site		
		There are two distinct groundwater aquife strata and the other within a layer of sup overlay the chalk. These two aquifers ar "aquitard" which limits movement betwee is largely recharged by rainfall and is dra lateral flow to surface water. However, t between the aquifers is less marked du "windows" between the Chalk and upper recharge there may be upward leakage surface water through the more permeats Finally, diurnal changes in the Chalk piezo the coast in response to the rise and fall of As a result of these influences. The s groundwater flooding.	ers under the site, one in the lower chalk berficial deposits (mainly Alluvium) which e separated by layers of clay forming an n the two aquifers. The Alluvium aquifer ained by evapotranspiration and through there are locations where the separation ue to the presence of more permeable aquifers. Under conditions of high winter from the Chalk to the upper aquifer and ble "windows" in the Superficial Deposits. cometric surface have been observed near of the tide level. ite is considered to be at high risk of	
		Tidal Groundwater Risk Zone (maximum risk)	Tidal Drainage Risk Zone (maximum risk)	
	Tidal Risk Zones	GW2 The Tidal Ground Water Risk Zones have Groundwater Flood mapping and as a re be higher than indicated due to the unique situated below the present-day tidal level a and previous studies have shown there groundwater levels. The east section of the site is located with is due to the site being located below the risk of surface water flooding in the future site that is situated within Tidal Drainage areas below the present-day tidal level ar the future. The western boundary of the s Drainage Risk Zone, Zone SW4. This is of present-day tidal level and at risk of flood event	SW4 ive been calculated using the JBA national result the tidal groundwater risk is likely to que local hydrogeology. The site is largely and is at high risk of groundwater flooding are is a link between tide levels and the ithin Tidal Drainage Risk Zone SW2. This he present-day tidal level but at a negligible re. To the west there is a section of the e Risk Zone SW3. This Zone correlates to and at risk from surface water flooding in e site is located within highest Tidal s due to it being located below the boding during the 1% AEP surface water	

The site is not at risk of flooding of reservoir flooding.

Reservoir

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JBA





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SHLAA / HELAA site reference		FB08229		
Site name		Shoreham Gateway Site		
	Bedrock Geology	The entire site's bedrock geology consists of White Chalk.		
	Superficial Geology	The entire site is overlain with alluvium deposits (clay, silt and sand) and as su is likely to experience perched groundwater in the soils over winter months.		
	Soils	The site has loamy and clayey soils of coastal flats with naturally high groundwater.		
	Groundwater Source Protection Zone	Approximately one third of the site in the north east is located within Groundwater Source Protection Zone 3, which is defined as an area around a supply source within which all the groundwater ends up at the abstraction point. The zone may extend some distance from the source point.		
	Historic Landfill Site	There are no historic landfill sites in the vicinity of the site.		
Requirement for drainage control and impact mitigation	Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could provide wider sustainability benefits to the site and surrounding area. Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development. Most source control techniques are likely to be appropriate. Background understanding of the groundwater conditions suggests that permeable paving may have to use non-infiltrating systems across the site due to the high risk of groundwater flooding. This must be confirmed via site investigations to assess the potential for infiltration. Whilst controlling run-off from proposed development must be addressed there is also a need to consider the effect of proposals on surface water flows such that predicted surface water flooding is not exacerbated at existing adjacent development.		

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SHLAA / HELA	A site reference	FB08229
Site name		Shoreham Gateway Site
	 Assessmer from the sit SuDS desi Statutory T 2015). Green infra runoff from Further det webpage surface wa LPA's requ 	nt of runoff should include allowances for climate change effects, and discharge rates te should not increase downstream flood risk. gn must follow West Sussex County Council policy, meet the Defra National Non- echnical Standards, and follow current best design practice (CIRIA C753 Manual astructure should be considered within the mitigation measures for surface water potential development. ails regarding Adur and Worthing Council requirements are available on the following <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms</u> . A tter drainage checklist is also available on this webpage. This clearly sets out the irements for avoiding pre-commencement conditions, or to discharge conditions.









SHLAA / HELAA site reference		WB08039		
Site name		Teville Gate, Railway Approach		
	OS Grid reference	TQ 14646 03288		
	Local Authority	Worthing Borough Council		
	Area	1.75 ha		
	Current land use	Vacant office buildings and cleared site		
	Proposed site use	Mixed use- to include 300 residential units		
	Flood risk vulnerability	More vulnerable		
Site details	Topography	Site topography Legend Site Boundary Levation Image: Site Boundary I		



SHLAA / HELAA site reference WB08039							
Site name		Teville Gate, Railway App	roach				
	Existing						
	watercourses	There are no existing watercourses located near the site.					
	Flood history	The Environment Agency's Recorded Flood Outline dataset records flooding occurring in 1980 as a result of poor drainage along Station Road and Newland Road, 300m to the east of the site. West Sussex County Council's recorded flood incidents dataset also records a number of incidents in these locations between 1960 and 1970 as well as in 2012.					
			Proportion of the site at risk	(
		(proportion reported a	are for the area of land occupie	ed by each flood extent			
		between larger or smal	ler return period events, and the pearest 1%. Areas	herefore not cumulative.			
		5% AFP	1% ΔFP	0.5% hot recorded)			
		0%	0%	0%			
Sources of flood risk	Fluvial	Available modelled data: The site is covered by the Teville Stream (Fluvial) 2012 Flood Modeller-TUFLOW model. The extent of the Flood Zones predicted by the flood model are also the extent of the actual flood risk, as there are no flood risk management features that change the risk. It should be noted that this model contains a surface water component, as such, the depth, velocity and hazard mapping outputs shown include the surface water element as well as the fluvial risk, this has not been included in the percentages above. Flood characteristics: The risk of flooding from fluvial sources is negligible for the site. The site is entirely located within Flood Zone 1. Proportion of site at risk (RoFSW)					
		between larger or smaller return period events, and therefore not cumulative.					
		3 3% AFP	ed to the hearest 1%. Areas • 1% ΔΕΡ	<0.5% not recorded) 0 1% ΔΕΡ			
		33%	15%	25%			
	Surface Water	33%15%25%Description of surface water flow paths:During the 3.3% AEP surface water event, areas in the southern half of t towards the south east corner of the site are predicted to be at risk of sur flooding (33%). For the 1% AEP event, there is an 15% increase in fl and flooding covers almost half of the site (48%). Increases in flood ex in the centre, south west corner and along the east boundary of the site increase of 25% is predicted to occur during the 0.1% AEP event. If predicted to occur for the entire southern half of the site with only areas north east and north west corner of the site at negligible risk.RoFSW takes account of building footprints so the flood risk may be a existing buildings on the site. It also only considers flood risk where rating is greater than 0.575.					

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SHLAA / HELA	A site reference	WB08039			
Site name		Teville Gate, Railway App	Teville Gate, Railway Approach		
		Proportion of site at risk in JBA Groundwater Map 1% AEP risk categories			
Groundwater		Depth below surface 0-0.025m	Depth below surface 0.025-0.5m	Total in highest risk categories	
		0%	100%	100%	
		The site is predicted to be	e at a high risk of groundwat	er flooding with groundwater	

		levels predicted to be between 0.025m and 0.5m from a ground surface 1% AEP groundwater flood event.		
		Tidal Groundwater Risk Zone (maximum risk)	Tidal Drainage Risk Zone (maximum risk)	
		GW0	SW3	
	Tidal Risk Zones	The site is entirely located within Tidal Groundwater Risk Zone 0. This is due to the site being located within an impermeable geological unit and therefore the groundwater is not thought to be tidally influenced. Approximately two thirds of the site is located within Tidal Drainage Risk Zone SW3. This is due to this area being located above the present-day tidal level but at risk during the 1% AEP surface water flood event. A small section of the site in the north east corner and along the eastern boundary is located within Tidal Drainage Risk Zone SW2. This correlates to an area above the present-day tidal level but at risk from surface water flooding in the future. A small section of the site in the north west is located within Tidal Drainage Risk Zone SW2 where it lies above the present tidal level and at a negligible risk from surface water flooding in the 1% AEP surface water event. Finally, the remainder of the site in the north west lies within Tidal Drainage Risk Zone SW0. The higher elevation of this land raises this area above the future tidal level and is therefore no longer at risk of being tidally influenced.		
	Reservoir	The site is not at risk of reservoir flooding		





SHLAA / HELAA site reference		WB08039				
Site name		Teville Gate, Railway Approach				
Flood risk management infrastructure	Defences	Defence Type		Stan	dard of Protection	Condition
		There are no defences within the vicinity of the site.				
	Residual risk	Culvert / structure blockage?		It is understood there may be a culverted watercourse which runs under the site.		
		Impounded water body failure?		The site is not at risk of flooding due to a reservoir breach.		
		Defence breach / overtopping?		The site is not at risk of flooding from a defence breach or overtopping.		
Emergency planning	Flood warning	The site is not covered by an Environment Agency Flood Alert or Flood Warning Area.				
	Access and egress	Dry access and egress could be available to the site during all surface water and fluvial events via Railway Approach located in the north west corner of the site.				
Climate	Climate change allowances for '2080s'	Proportion of site at 1% AEP fluvial flood risk				
		River Basin District	Present o	lay	Flood Zone 2 as a proxy for clim change	
		South East	0%	0%		
	Implications for the site	Using Flood Zone 2 as a proxy for climate change shows that the site is not sensitive to the impact of increased flows.				
Change	Impact of climate change on risk from surface water	Proportion of site at 1% AEP surface water flood risk				
		Present day	+20% rai uplift	nfall	+30% rainfall uplift	+40% rainfall uplift
		48%	52%		53%	57%
	Implications for the site	There is a small increase in flood extent during the 1% AEP surface water event for the plus 20%, 30% and 40% climate change events. These increases are located to the north west and south west of the site. However, these increases do not reach the 0.1% AEP surface water extent. Therefore, the site will be at a moderately higher risk from surface water flooding in the future.				

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A site reference	WB08039	
	Teville Gate, Railway Approach	
Bedrock Geology	The majority of the site's bedrock geology consists of London Clay Formation (clay, silt and sand). The south of the site is formed of Lambeth Group (clay, silt and sand).	
Superficial Geology	The site is overlain with Raised Beach (sand and gravel) superficial deposits.	
Soils	The site has freely draining slightly acid loamy soils.	
Groundwater Source Protection Zone	The site is not located within a Groundwater Source Protection Zone.	
Historic Landfill Site	The site is not situated near a historic landfill site.	
Broad scale assessment of possible SuDS	Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could provide wider sustainability benefits to the site and surrounding area. Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development. There are numerous foul and surface water sewers crossing the site which are likely to have an impact on surface water drainage design. Most source control techniques are likely to be appropriate. Mapping suggests that permeable paving may have to use non-infiltrating systems given the possible risk from groundwater flooding (medium to high across the site). This must be confirmed via site investigations to assess the potential for infiltration. Whilst controlling run-off from proposed development must be addressed there is also a need to consider the effect of proposals on surface water flows such that predicted surface water flooding is not exacerbated at existing adjacent development. Given the high-density nature of the site, use of SuDS is recommended – urban sites should not preclude the use of SuDS. Infiltration techniques may be appropriate. Mapping suggests a medium to high risk of groundwater flooding and underlying soils may be permeable. Further site investigation must be carried out to assess potential for drainage by infiltration. Mapping suggests that the site slopes make it possible to consider most forms of detention. A liner maybe required due to the potential groundwater flooding on the site. All forms of conveyance are likely to be appropriate. Where the slopes are >5%	
	All forms of conveyance are likely to be appropriate. Where the slopes are >5% features should follow contours or utilise check dams to slow flows. A liner maybe required to prevent the ingress of groundwater.	
	A site reference Bedrock Geology Superficial Geology Soils Groundwater Source Protection Zone Historic Landfill Site Broad scale assessment of possible SuDS	

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SHLAA / HELAA site reference		WB08039	
Site name		Teville Gate, Railway Approach	
	 Assessment Efforts should not SuDS desite Statutory T 2015). 	nent of runoff should include allowances for climate change effects. nould be made to limit runoff to greenfield rates and discharge rates from the site ot increase downstream flood risk. esign must follow West Sussex County Council policy, meet the Defra National Non- / Technical Standards, and follow current best design practice (CIRIA C753 Manual	
	Green infra runoff from	Freen infrastructure should be considered within the mitigation measures for surface water unoff from potential development.	
	Further det following w A surface v LPA's requ	ails regarding Adur and Worthing Council requirements are available on the rebpage <u>https://www.adur-worthing.gov.uk/planning/applications/submit-fees-forms</u> . water drainage checklist is also available on this webpage. This clearly sets out the irements for avoiding pre-commencement conditions, or to discharge conditions.	



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Site name	Teville Gate, Railway Approach	Adur and Worthing Level 2 Strategic Flood Risk Assessment Site Summary Sheet mapping	ADUR & WORTHING		
Site area (ha)	1.75		COUNCILS	consulting	

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